

# **Geotechnical Impact Assessment Study**

#### Submitted to:

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# **Executive Summary**

Golder Associates Africa (PTY) Ltd (Golder) washas been appointed by Zitholele Consulting (Pty) Ltd (Zitholele) to undertake preliminary geotechnical investigations for the waste disposal facilities of three waste streams to be generated by the operation of the Flue Gas Desulfurization (FGD) at Medupi power station at Lephalale, Limpopo.

The Scope of Work was to undertake a preliminary geotechnical assessment of the Ash Disposal Facility on the preferred Alternative Site 13. The waste disposal infrastructures include a gypsum landfill and lagoons to dispose salts and sludge.

Alternative Site 13 is situated at the western portion of the Medupi power plant. Eight intrusive test pits were excavated in the area of Alternative Site 13.

A total of 8 test pits were excavated using a TLB to a median depth of 2m. The test pits intersected topsoil overlying alluvial non-cohesive material, with weathered sandstone intersected in only one test pit at a depth of 1.7m.

Recommendations have been provided on excavatability, engineering use, shear strength, shallow foundations and slope stability.

The scope of works was revised after the fieldwork was complete to include professional geotechnical opinion on additional scope items of infrastructure using supplied information.

No significant geotechnical hazards or fatal flaws were identified. All the geotechnical considerations mentioned can be mitigated in the design of the facility. Geotechnical investigations will be required as the project proceeds into design.





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#### **ESKOM MEDUPI FGD**

#### 1.0 INTRODUCTION

Golder Associates Africa (PTY) Ltd (Golder) was appointed by Zitholele Consulting (Pty) Ltd (Zitholele) to undertake preliminary geotechnical investigation for the waste disposal facility of the three waste streams to be generated by operation of the Flue Gas Desulfurization (FGD) at Medupi power station at Lephalale, Limpopo.

This is a report of the geotechnical impact assessment findings.

#### 2.0 OBJECTIVES

The objectives of the geotechnical investigation and impact assessment were as follows:

- Undertake a site visit, taking cognisance of the geotechnical sensitivity of the site;
- Undertake a review of existing information and conceptual plans of the study area;
  - § founding material
  - § waste dump material (using previous or supplied reports, published, empirical or other data sources)
  - § liner system (using previous or supplied reports, published, empirical or other data sources)
- Provide mitigation measures to prevent and/or mitigate any impacts that may occur due to the proposed project;
- Assess groundwater conditions encountered during the investigation and their potential engineering impact on the proposed development.
- Provide a Geotechnical Impact Assessment commenting on any visible or readily identifiable geotechnical issues exposed during the investigations which may affect the proposed development.

#### 3.0 SCOPE OF WORK

The scope of works was initially to assess the footprint of the ash dump waste disposal facility extension, but was revised after the fieldwork was complete to include the following:

- Construction and operation of a rail yard/siding to transport Limestone from a source defined point via the existing rail network to the Medupi Power Station and proposed rail yard / siding. The rail yard infrastructure will include storage of fuel (diesel) in above ground tanks and 15m deep excavation for tippler building infrastructure;
- 2) Construction and operation of limestone storage area, preparation area, handling and transport via truck and conveyor to the FGD system located near the generation units of the Medupi Power Station;
- 3) The construction and operation of the wet FGD system that will reduce the SO<sub>2</sub> content in the flue gas emitted;
- 4) Construction and operation of associated infrastructure required for operation of the FGD system and required services to ensure optimal functioning of the wet FGD system. The associated FGD infrastructure include a facility for storage of fuel (diesel), installation of stormwater infrastructure and conservancy tanks for sewage;
- 5) The handling, treatment and conveyance of gypsum and effluent from the gypsum dewatering plant. Disposal of gypsum on the existing ADF is not included in the current EIA application and will be addressed in the ADF WML amendment application.
- 6) Pipeline for the transportation of waste water from the gypsum dewatering plant and its treatment at the WWTP that will be located close to the FGD infrastructure within the Medupi Power Station;



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- 7) Construction and operation of the WWTP;
- 8) Management, handling, transport and storage of salts and sludge generated through the waste water treatment process at a temporary waste storage facility. In terms of the EIA process impacts related to the management of salts and sludge will be considered in the EIR. However, licencing of the storage activity and requirements relating to the waste storage facility will be assessed in the WML registration application process.
- 9) The transportation of salts and sludge via trucks from the temporary waste storage facility to a final Waste Disposal Facility to be contracted by Eskom for the first 5 years of operation of the FGD system. Long term disposal of salts and sludge will be addressed though a separate independent EIA process to be commissioned by Eskom in future.
- 10) Disposal of gypsum together with ash on the existing licenced ash disposal facility (ADF), with resulting increase in height of the ADF from 60m to 72m.

#### 4.0 ASH DISPOSAL FACILITY SITE DESCRIPTION

# 4.1 Site Description

The Medupi power plant is located approximately 25km west of the town of Lephalale in Limpopo. Alternative Site 13 is within the bounds of the plant. The sites were accessed by gravel tracks that lead off the main road from the town of Lephalale.

The site layout map showing the location of the preferred Ash Disposal Facility (Alternative Site 13) in relation to alternative Sites initially considered is presented in Figure 1 below.



Figure 1: Site layout





The general topography of the site comprises large level plains with wild animals (bucks and warthogs) seen on the sites during the investigation. Construction is currently taking place in an area adjacent to Alternative Site 13, but it is a progressing development.

The bush comprises a thick concentration of trees, shrubs and grass interspersed with large trees. Movement through the bush by vehicles is generally difficult, with some tracks used by Eskom. The site investigation team relied on tracks formed by the excavator to navigate the site. No major rivers or water bodies were observed.

Photographs of the sites, taken during the geotechnical fieldwork, are included below:

#### **Alternative Site 13**



Figure 2: Construction activity seen on site



Figure 3: Typical test pit investigated on site (shallow refusal)



Figure 4: Typical material excavated from site



Figure 5: Typical vegetation on site



# 4.2 Nature of the Investigation

#### 4.2.1 Field Investigation over the ADF Footprint

Geotechnical fieldwork was undertaken from 3 November to 6 November 2015 and comprised the following:

Mechanical excavation, using an excavator provided by Golder, of 8 test pits at pre-determined positions. The pre-determined GPS positions of each test pit were located on site using a handheld GPS unit accurate to within about 5 m. Test pits were excavated to depths of approximately 3 m below natural ground level (NGL). Test pits were profiled according to the method of prescribed in SANS 633. Retrieval of representative soil samples was undertaken from some test pits for laboratory testing purposes.

The approximate fieldwork positions are presented in the figure attached in Appendix B.

### 4.2.2 Laboratory Testing

Soil samples were collected from the test pits and transported to a specialist laboratory in Johannesburg. The soil samples were submitted for testing to determine fundamental geotechnical engineering properties of the materials including soil classification, material and compaction properties.

The following sample types and number of samples were retrieved during the investigations:

Table 1: Samples types and quantities

Source	Disturbed Soil	Disturbed Soil (Bulk)
Test Pits	5	1

# 4.3 Geology and Soil Profile

#### 4.3.1 Geology and Climatic Value

The published 1:250 000 Geological Map of Elisras (1993) shows the proposed site area is underlain by coarse grained brown sandstone and conglomerates of the Mogalakwena Formation, Waterberg Group. In the area of interest, thick profiles of sandy Alluvium can be expected.

The area is classified as having a climatic N-value (Weinert, 1980) of almost 5, which indicates that both chemical weathering and mechanical weathering are likely. From the description of the geology of the area it can be expected that residual soils are generally shallow and transported soils vary greatly in thickness.

#### 4.3.2 Soil Profile

A total of 8 test pits were excavated within the footprint of Alternative Site 13. This is a deviation from the original 9 test pits that where initially planned. Test Pit 7 was inaccessible during the field investigation.

The excavated test pits intersected a relatively uniform near surface ground profile across all alternative sites. The intersected ground profile within the 3.0 m from NGL generally comprises four successive horizons. These general horizons have been described below for interpretive purposes.





Table 2: Typical Soil Profiles the Alternative ASF Site

Topsoil	Dry to slightly moist, brown to brownish grey, very loose becoming loose with increasing depth, intact, fine grained; SILTY SAND – <i>Topsoil</i> with some roots and organic material.
Transported Sand (Alluvium)	Slightly moist, reddish brown, medium dense to dense with increasing depth, intact to slightly shattered, fine to medium grained; slightly SILTY CLAYEY SAND to SILTY SAND – <i>Transported (Alluvium)</i> .
Transported Gravel	Dry, brown speckled black, dense becoming dense with depth, homogenous, medium to coarse grained, matrix supported, blocky, rounded, smooth to granular, hard rock; weakly cemented GRAVEL, excavates as gravel – Transported
Sandstone	This material was only encountered in MTP9 at depth 1.1. Reddish brown, highly weathered, fine grained, soft rock, in situ weathered SANDSTONE excavated as rounded, subangular gravel.

**Table 3: Ground Profile underlying the ADF Footprint Alternative Site 13** 

Took Bit ID	Bouth (m)	Depth to the bi	Depth to the base of indicated horizon (or base of excavation, where indicated) (m)							
Test Pit ID	Depth (m)	Topsoil	Transported Sand	Transported Gravel	Sandstone					
Alternative Site 13										
MTP1	1.1	0.2	1.1*	-	-					
MTP2	1.2	0.4	1.2	-	-					
MTP3	1.1	0.4	1.1	-	-					
MTP4	1.4	0.4	1.3	1.4*	-					
MTP5	1.6	0.5	1.6*	-	-					
MTP6	2.5	0.4	1.5	2.5*	-					
MTP8	1.5	0.5	1.0	1.5*	-					
MTP9	1.7	0.4	1.1	-	1.7*					

<sup>\*</sup>Base of excavation

The area of the proposed development appears to be covered in its entirety by relatively thick alluvial sand. This surface horizon was found to be dry and was up to 2 m thick in some test pits.

Cemented gravel material was found is some test pits. This material is interpreted to be also be of transported origin.

Of the 8 test pits, weathered rock was only encountered in MTP9.



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#### 4.3.3 Moisture Condition

No seepage or groundwater was encountered in any of the test pits, which reached a maximum depth of 2.5m.

# 4.4 Laboratory test results

#### 4.4.1 Foundation Indicators

**Table 4: Foundation Indicator test results** 

			Soil C	ompos	sition				G LL (%)	PI LS (%)	16	
Test Pit ID	Depth (m)	Material type	Clay (%)	Silt (%)	Sand (%)	Gravel (%)	GM	SG				usc
	Alternative site 13											
MTP1	0.4 – 1.1	Transported	15	12	72	1	1.01	2.598	22	11	4	SC
MTP2	1.2	Transported	19	9	63	9	1.21	2.825	26	12	6.5	SC
MTP3	0.4 – 1.1	Transported	12	7	71	10	1.41	-	24	8	4	SC
MTP5	0.5- 1.0	Transported	14	9	74	3	1.14	2.658	20	10	3.5	SC
MTP8	0.5- 1.0	Transported	9	8	80	3	1.18	2.852	NP	NP	NP	SM

Legend:

GM = Grading Modulus

SG = Specific Gravity

LL = Liquid Limit

PI = Plasticity Index

LS = Linear Shrinkage

SC = Clayey Sand

SM = Silty Sand

#### Table 4 indicates that:

The **Transported** soil of **Alternative Site 13** comprises mostly of sand (63% to 80%) and some fraction of clay and silt. The material can be classified as Clayey Sand (SC) and Silty Sand (SM) according to the Unified Soil Classification (USC). The Plasticity Index of the material is low (8% to 12%), this together with the low clay contents are indicative of a low plastic material with a low potential expansiveness according to the method proposed by Van der Merwe (Reference 5).





### 4.4.2 Compaction Characteristics

**Table 5: Compaction Test Results** 

Test pit ID	Depth Material (m) Type	OMC (kg/m (%) Swell (%)		Soaked CBR at various ratios of MDD compactive efforts						TRH 14 Classificati		
		.,,,,	(/-)	<sup>3</sup> )	(7-5)	90%	93 %	95 %	97 %	98 %	100 %	on
	Alternative site 13											
MTP3	0.4- 1.1	Transported	8.4	2115	0.2	9	13	18	21	21	23	G9

Legend

OMC = Optimum moisture content

MDD = Maximum dry density (Modified AASHTO)

Swell = Soaked at 100% Mod AASHTO compaction

TRH = Material classification according to TRH14 guidelines

CBR = California Bearing Ratio

The **Transported** sand for **Alternative Site 13** has a moderate dry density of 1961 kg/m³ and 2115 kg/m³ and a relatively high moisture content of 11.3 and 8.4% respectively. The swell is low at 1.4% and 0.2% respectively. The CBR value of 93% and 95% compaction effort are relatively low, but typical for sandy material. Accordingly to the TRH 14, the material can be classified as G9 material.

#### 4.4.3 Falling Head Permeability

**Table 6: Permeability Test Results** 

Test Pit ID	Depth	Moisture	Content	Dry (Kg/m3)	density	Coefficien	t of Permeak	oility (m/s)		
Test Fit ID	(m)	Before		Range		A.v.a.v.a.v.a				
		Test (%) Test (%) Tested	Tested	Minimum	Maximum	Average				
	Alternative Site 13									
MTP 2	0.4-1.2	8.7	14.7	1395	1760	1.9× 10 <sup>-7</sup>	2.5× 10 <sup>-7</sup>	2.1× 10 <sup>-7</sup>		
MTP 8	0.5-1.0	4.3	14.5	1427	1743	5.3× 10 <sup>-7</sup>	6.9× 10 <sup>-7</sup>	6.2× 10 <sup>-7</sup>		

The laboratory test results show a relatively low permeability. This is due to the moderate content of clay and silt in the Transported material. Should the in-situ transported soil be used in the construction of embankments or liner material, a permeability of 1×10<sup>-7</sup> m/s can be expected when compacted to 95% Mod AASHTO effort.





#### 4.4.4 pH and Conductivity

Table 7: pH and Conductivity test results

Test Pit ID	Depth (m)	pH-value	Conductivity mS/cm						
Alternative Site 13									
MPT1	0.40-1.10	5.74	0.3300						
MTP5	0.50-1.00	4.56	0.0897						

The pH and conductivity of soil is generally determined to get an indication of the potential corrosiveness of the soil. The pH of a soil gives an indication of the acidity of the soil. As a general guideline Evans (Reference 8) noted that corrosion may take place in soil with a pH of less than 6 and that should the pH be less than 4,5, the problem may be serious. It should however be noted that a low pH value is not necessarily an indication of serious corrosiveness as the pH of the surrounding soil will generally start to rise as soon as corrosion starts.

Corrosion is an electrochemical process whereby metals are changed and electrical energy is released. The conductivity of the soil therefore has a profound influence on the rate of corrosion of buried metallic objects.

The conductivity results are generally very corrosive (0.25 - 0.5 mS/cm) corrosion classification accordingly to the method proposed by Duligal E (Reference 7) with the exception MTP 5 in Alternative site 13. The pH value obtained are indicative of material with medium potential to corrode.

# 4.5 Geotechnical Impact Assessment

An evaluation of the impact of geotechnical conditions on Alternative Site 13 is presented in the sections below.

#### 4.5.1 Excavation Class

Excavation Class was assessed according to SANS 1200D: Earthworks Specification (Reference 4) for the areas investigated by means of test pits and at the positions of the test pits, based on a summary of the Excavation Class provided below:

- Soft excavation, which is possible using conventional light earthmoving equipment (TLB or similar)
- Intermediate excavation, which will require heavier plant (tracked excavator) or pneumatic tools to break up material prior to excavation
- Hard excavation, requiring drill-and-blast operations, and boulder material potentially requiring localised drill and blast operations and heavy plant for removal thereof.
- \* Boulder Class, requiring individual drilling and blasting in order to be loosened

The interpreted excavation class for the sites are as follows:





**Table 8: Evaluation of Excavation Class** 

	Approximate Average Depths (m)						
Site	Soft Excavation Class	Soft to Intermediate Excavation Class	Boulder and Hard Rock Excavation Class				
Alternative Site 13	0 – 0.4m	0.6 – 2.5m	Approximately 2.5m+				

Since rock was encountered from relatively shallow depths elsewhere, variation to the above can be anticipated between points of excavation. An allowance should therefore be made for boulder and hard excavation in areas not inspected, or where TLB refusal was encountered.

#### 4.5.2 Engineering Use

A geotechnical evaluation of the engineering properties of the materials encountered on site, together with an appraisal of the suitability of the materials for engineering use and construction based on Wagner (Reference 6), is tabulated below:

Table 9: Preliminary engineering use and construction suitability

Material Type	USC Range	Workability	Homogeneous Embankment Earth Fill	Road Pavement Fill	Liner material
Transported/ Residual	SC to SM	Fair - good	Good	Poor	Fair
Transported	SC	Good	Good	Poor	Fair

USC - Unified Soil Classification

Transported sand was found within the footprint of Alternative Site 13. This material is expected to be found in abundance, although intermediate excavation measures will have to be employed to access this material. This material can be used in embankment fills. The laboratory test results revealed that there is a considerable amount of cohesive material present in the transported material and can, therefore, be appropriate for use as a lining material. Imported clay or a synthetic liner may be needed in addition to the transported material for the construction of the liner for the waste disposal facilities.

#### 4.5.3 Shear Strength

The effective shear strength properties of the in situ soils have been estimated based on the laboratory test results, engineering experience and available literature, as follows:

Table 10: Preliminary effective shear strength parameters

Material Type	Estimated effect strength parameters	tive in situ shear eters	Estimated effective shear strength parameters of material compacted to at least 95% of MDD		
	c' (kPa)	φ' (degrees)	c' (kPa)	φ' (degrees)	
Transported	0 26 - 30		0	32	

MDD: Maximum dry density





#### 4.5.4 Shallow Foundations

The following recommendations on bearing capacity should be considered preliminary and subject to further investigation and analysis as the design proceeds.

Design of shallow strip and pad footings are considered suitable on properly compacted alluvial sand and gravel which covers the majority of Alternative Site 13. These should generally be designed for light structures (loads not exceeding 100kPa), subject to depth of embedment and minimum footing width requirements. The suitability of the site for deep pad foundations in order to found statically loaded heavy structures is considered suitable up to about 300kPa, subject to similar requirements as per lightly loaded structures above. Note that final recommendations need to be reviewed once the details of the waste disposal facilities and their accompanying loads are known. Further ground intrusive investigations (core drilling, geophysical investigation and SPT) will be required.

#### 4.5.5 Lateral Stability of Excavation

The lateral stability of excavation side walls are not expected to be problematic. We recommend the following preliminary slopes:

- Permanent cut slopes in non-cohesive sand and gravel: 1 Vertical (V): 3.0 Horizontal (H)
- Temporary cut slopes in sand and gravel: 1V: 2H
- Permanent embankment fill slopes in non-cohesive sand and gravel: 1V: 2.5H

# 4.5.6 Summary of Geotechnical Impact Assessment





Table 11: Geotechnical Impact Assessment of Alternative Site 13

Activity	Nature of Impact	Impact type	Extent	Duration	Potential Intensity	Likelihood	Rating	Mitigation	Interpretation
Excavation Class	Excavation Class was assessed according to SANS 1200D: Earthworks Specification for the areas investigated by means of test pits and at the positions of the test pits.	Existing	2	1	2	1	6 - Moderate	The use of mechanical equipment or blasting to excavate below 2.5m	A contingency should be allowed from depths of 2.5m below NGL
Engineering Use	A geotechnical evaluation of the engineering properties of the materials encountered on site, together with an appraisal of the suitability of the materials for engineering use and construction based on Wagner (1957)	Existing	2	1	2	0.75	5.75 - Moderate	The material is good for use as an embankment fill but better material would need to be imported to construct the roads leading to the facility.	Gravel material (G6 or better) and clay material may need to be imported in the construction phase to use as liner and construction material
Shear strength	The effective shear strength properties of the in situ soils have been estimated based on the laboratory test results, engineering experience and available literature	Residual	2	2	2	0.75	6.75 - Moderate	The shear strength of the material is quite low and would need to be further tested. The shear strength can be improved by stabilising the material with lime or cement.	The shear strength is appropriate for lightly loaded structures but would need to be improved for heavy structures
Bearing Capacity	Bearing capacity should be considered preliminary and subject to further investigation and analysis as the design proceeds.	Existing	2	5	8	1	16 - High	Bearing capacity for shallow founding is suitable for lightly loaded structures, deep foundations would be necessary for heavy structures (bigger than 300kPa)	The bearing capacity is appropriate for lightly loaded structures but would need to be improved for heavy structures
Lateral Stability	The lateral stability of excavation side walls are not expected to be problematic. We recommend the following preliminary slopes:	Existing	2	2	4	1	9- Moderate	1V:2.5H excavations are recommended for permanent slopes while 1V:2H excavations are recommended for temporary slopes.  Benching of slopes may be required.	The lateral stability of excavation side walls are not expected to be problematic if mitigation measures are implemented



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#### 5.0 FGD SYSTEM WITHIN THE MEDUPI POWER STATION FOOTPRINT

We provide a brief geotechnical assessment of the FGD system within the Power Station footprint using only supplied information, as this area was not included within our original scope of works.

Based on the Golder report reference 12087-8856-1 entitled: *Medupi Power Station: Shallow Groundwater Study,* and dated June 2009 (Reference 2), a number of percussion (hydrological) boreholes were drilled in and around the Power Station footprint. The location of these boreholes are provided in the figure in Appendix B.

The profiles presented in the report indicates the following ground conditions:

- The site is underlain by a sequence of pebbles, weathered quartzitic conglomerate with fresh variously fractured quartzitic conglomerate at depth.
- The conglomerate is interbedded with bluish grey siltstone (bands). The drilling has shown the siltstone forms discontinuous layers of up to 50cm thick but mostly about 20cm thick.
- Generally surface weathering to shallow depth (<5m) occurs. In some boreholes a second fractured and associated weathered zone is observed and is normally found between 7 14m.
- Some boreholes (e.g. GA001, GA003, GA004, GA005 and GA014) have no surface weathering.
- Boreholes in the extreme north or west, show the presence of deep weathering, up to 21m.
- Water strikes were made in 14 of the 35 boreholes at depths between 6 and 10.5m below surface.

Based on this information, the following brief comments are provided:

- The site is mainly underlain by quartzites, shale, sandstones and conglomerates. Soils and weathered and fractures rock are present to depths typically varying from 10 to 15m, below which the soils become relatively fresh.
- Standard foundation systems are expected to be applicable, comprising generally shallow foundations.
- Excavatability is expected to be soft to intermediate, with hard rock class (drill and blast) for excavation in moderately weathered or harder rock (location dependent, but generally below about 5m depth).

# 6.0 RAILWAY YARD AND LIMESTONE AND GYPSUM HANDLING FACILITIES

We provide a brief geotechnical assessment of the Railway Yard and Limestone and Gypsum Handling facilities using supplied information only, as this area was not included in our original scope of works.

Based on the supplied Rockland Geocscience report (Reference 3), the following is noted:

The investigation comprised the excavation of test pits and geophysical surveys over the site. Test pit TP7 and TP23 are located near to the proposed railway yard. These encountered medium dense silty sand to between 1.1m and 1.8m, underlain by dense gravel to between 1.5m and 2.4m, underlain by very soft rock quartzite in TP7, with TLB refusal at 1.8m on medium hard rock quartzite in TP7, and refusal on hardpan ferricrete at 2.4m in TP23. The geophysical continuous surface wave (CSW) survey CSW4 conducted adjacent to TP7 interpreted a shear wave velocity profile with highly weathered (quartzite) rock below about 1.5m, becoming moderately weathered below about 4m, then slightly weathered below about 8.5m.

Based on the Golder report reference 12087-8856-1 entitled: *Medupi Power Station: Shallow Groundwater Study* and dated June 2009, a number of boreholes were drilled in and around the Power Station footprint. We have reviewed the two boreholes closest to the proposed Railway Yard, GA026 and GA027, which are located about 800 to 1000m away. The water level measured in GA026 was dry, and in GA027 was 2.6m below surface.



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#### **ESKOM MEDUPI FGD**

Borehole GA026 indicates slightly and moderately weathered conglomeratic quartzite in zones below 3.5m depth, becoming fresh from 14.5m depth, whilst GA027 indicated the boundary between slightly to moderately weathered quartzite and fresh quartzite at 16.5m.

The Limestone Offloading Facility below the railway yard is proposed to be 15m in depth. Based on the above, the following is interpreted:

Hard rock (drill and blast excavation) will be required from a depth of about 2m.

Dependent on the thickness of the surficial soils and any fill materials over the area, a contingency allowance should be made for encountering rock during the installation of such services or shallow foundations, where hard rock excavation (hydraulic rock hammer or drill and blast) may be necessary.

Standard footing systems such as shallow pad and strip footings are expected to be applicable for the area.

Deep excavations are expected to require reinforcement and/or stabilisation, particularly at shallow depths. Dependent on the quality of the rock and degree of fracturing, the lower half of the 15m deep excavation may potentially be unreinforced and unstabilised. Core orientated geotechnical drilling and associated structural analysis of the ground will be required prior to design to test for this design solution.

Groundwater can be expected from a shallow depth in the excavation. The volume of water seepage is expected to be relatively low, and reducing as the excavation proceeds into less fractured rock.

#### 7.0 CONCLUSION

A total of 8 test pits were excavated at the site of the ash disposal facility using a TLB to a median depth of 2m. The test pits intersected topsoil overlying alluvial non-cohesive material, with weathered sandstone intersected in only one test pit at a depth of 1.7m. Recommendations have been provided on excavatibility, engineering use, shear strength, shallow foundations and slope angles at the ash disposal facility.

The ground conditions at the site of the FGD system within the plant site were assessed using percussion (hydrological) boreholes. Based on this, weathered rock is expected from about 5m depth. We note that this depth will be highly variable, and further testing will be needed to confirm the ground conditions at each item of infrastructure.

The ground conditions at the site of the Rail Yard and the Limestone and Gypsum Offloading Facility were assessed using percussion (hydrological) boreholes about 800m to 1000m away from the area, as well as test pits and geophysics conducted near to the area. Based on this, weathered rock and/or hardpan ferricrete can be expected from about 2m depth. This is underlain by weathered quartzite, becoming slightly weathered to fresh from about 8m.

No significant geotechnical hazards or fatal flaws were identified. All the geotechnical considerations mentioned can be mitigated in the design of the facility. Significant further investigations will be required for all items of infrastructure as the design proceeds.

#### 8.0 REFERENCES

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# **YA**

#### **ESKOM MEDUPI FGD**

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- 17) Duligal E. 1996: "Significance of Soil Resistivity on Corrosivity".
- 18) Evans, U R. 1977: "The Corrosion and Oxidation of Metals: Scientific principles and practical applications. Edward Arnold (Publishers ) Ltd.

#### 9.0 DOCUMENT LIMITATIONS

Your attention is drawn to the attached: "Document Limitations". The statements presented in this document are intended to advise you of what your realistic expectations of this report should be, and to present you with recommendations on how to minimise the risks associated with the groundworks for this project. The document is not intended to reduce the level of responsibility accepted by Golder Associates, but rather to ensure that all parties who may rely on this report are aware of the responsibilities each assumes in so doing.

**GOLDER ASSOCIATES AFRICA (PTY) LTD.** 

Simon Owens-Collins

Senior Engineering Geologist

KM-SOC/SOC/soc

Reg. No. 2002/007104/07

Directors: SA Eckstein, RGM Heath, SC Naidoo, GYW Ngoma

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# **APPENDIX A**Document Limitations





#### **DOCUMENT LIMITATIONS**

#### **DOCUMENT LIMITATION**

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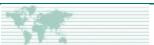
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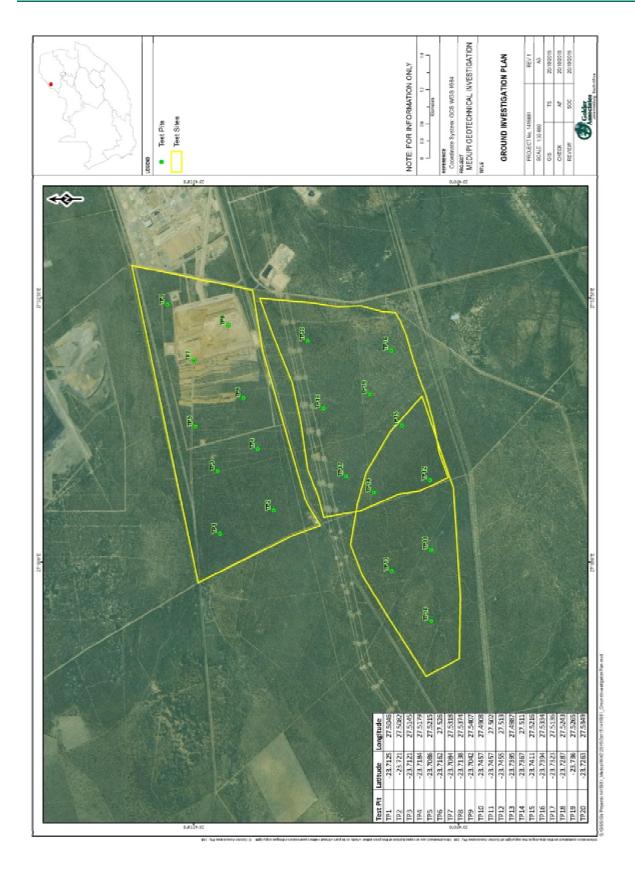


# **APPENDIX B**

**Figures** 

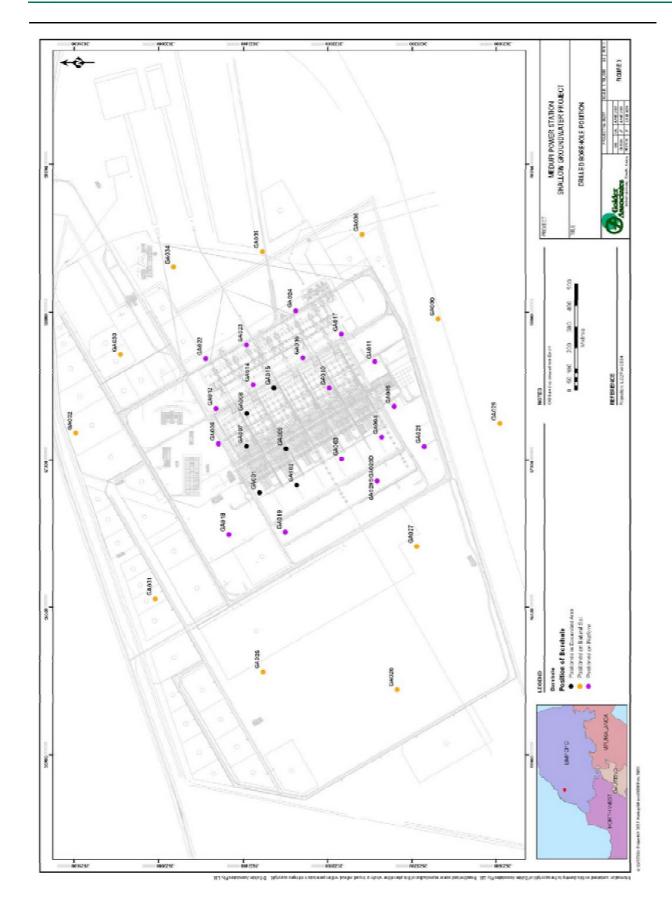






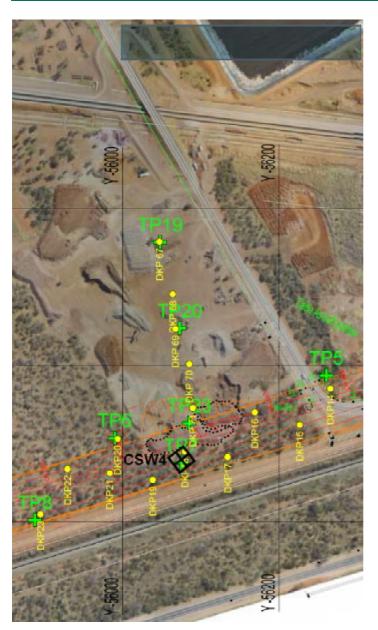












Excerpt from Rockland Geocscience report reference RG014/169/Rev0 over area of proposed Railway Yard





# **APPENDIX C**

**Profile logs** 





CLIENT: ESKOM

PROJECT: Medupi Geotechnical investigation

LOCATION: Lephalale, Limpopo

PROJECT NO: 1415881

HOLE No: MPT2

X COORD: 27.50 Y COORD: -23.71 DATUM: WGS84

ELEVATION:

Page 1 of 1

Scale	Legend	Depth	Description	
0.5 -		0.40	TOPSOIL.	
-				End of log

NOTES: 1: Medium to hard excavation

2: Refusal at 1.2m (very dense material)

3: No water

4: Side walls stable

5: Sample at 0.4-1.2m (FI, Permeability)

CONTRACTOR: Labante 5 (PTY) Ltd

MACHINE: CAT 48.8E TLB

DATE EXCAVATED: 2015/11/06

DATE PROFILED: 2015/11/06

PIT LxB: 2x 0.8 m

PROFILED BY: KM/DM

FILE REF: ESKOM.GPJ

CHECKED BY:

Golder Associates

Golder Associates Africa (PTY) Ltd

Buildinjg 1, Maxwell Office Park Waterfall City, Midrand 1685

Telephone: +27-11-254-4800

Fax: +27-86-582-1561



CLIENT: ESKOM

PROJECT: Medupi Geotechnical investigation

LOCATION: Lephalale, Limpopo

PROJECT NO: 1415881

HOLE No: MPT3

X COORD: 27.51 Y COORD: -23.72 DATUM: WGS84

ELEVATION:

Page 1 of 1

Scale	Legend	Depth	Description	
-		0.00	TOPSOIL.	
0.5 —		_0.40	Dry, reddish brown, <u>dense to very dense</u> , intact, <u>fine sand</u> .  TRANSPORTED.	
1.0 —				
1.5 —		1.70		
_		1.70		End of log

NOTES: 1: Medium excavation

2: Sample at 0.4-1.1m (CBR-Mod)

3: No water4: Side walls stable5: Refusal at 1.7m

CONTRACTOR: Labante 5 (PTY) Ltd MACHINE: CAT 48.8E TLB PIT LxB: 2x 0.8 m FILE REF: ESKOM.GPJ DATE EXCAVATED: 2015/11/06 DATE PROFILED: 2015/11/06 PROFILED BY: KM/DM CHECKED BY:





CLIENT: ESKOM

PROJECT: Medupi Geotechnical investigation

LOCATION: Lephalale, Limpopo

PROJECT NO: 1415881

HOLE No: MTP1

X COORD: 27.51 Y COORD: -23.71 DATUM: WGS84

ELEVATION:

Page 1 of 1

Scale	Legend	Depth	Description
-		0.20	Dry, brown, loose to medium dense, intact, fine sand.  TOPSOIL.  Dry, brown, dense, intact, fine sand.  TRANSPORTED.
0.5 -			
-			End of log

NOTES: 1: Medium excavation

2: No water3: Side wall stable

4: Hole terminated at 1.1m (slow excavation) 5: Sample at 0.4 - 1.1 m (FI and permeability)

CONTRACTOR: Labante 5 (PTY) Ltd MACHINE: CAT 48.8E TLB PIT LxB: 2x 0.8 m

FILE REF: ESKOM.GPJ

DATE EXCAVATED: 2015/11/06 DATE PROFILED: 2015/11/06 PROFILED BY: KM/DM CHECKED BY:





CLIENT: ESKOM

PROJECT: Medupi Geotechnical investigation

LOCATION: Lephalale, Limpopo

PROJECT NO: 1415881

HOLE No: MTP4

X COORD: 27.52 Y COORD: -23.72 DATUM: WGS84

ELEVATION:

Page 1 of 1

Scale	Legend	Depth	Description
-		0.00	TOPSOIL.
0.5 —		_0.35	Dry, reddish brown, medium dense to dense, intact, fine sand.  TRANSPORTED.
1.0		<u>1.25</u>	Dry, brown speckled black and red, dense, homogeneous, ferruginised gravel excavated as gravel.  TRANSPORTED (PEBBLE MARKER?).
1.5 —		1.40	End of log

NOTES: 1: No water

2: Side walls stable

3: No sample taken

4: Medium to hard excavation

5: Refusal at 1.4m

CONTRACTOR: Labante 5 (PTY) Ltd MACHINE: CAT 48.8E TLB PIT LxB: 2x 0.8 m FILE REF: ESKOM.GPJ DATE EXCAVATED: 2015/11/06
DATE PROFILED: 2015/11/06
PROFILED BY: KM/DM
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CLIENT: ESKOM

PROJECT: Medupi Geotechnical investigation

LOCATION: Lephalale, Limpopo

PROJECT NO: 1415881

HOLE No: MTP5

X COORD: 27.52 Y COORD: -23.71 DATUM: WGS84

ELEVATION:

Page 1 of 1

Scale	Legend	Depth	Description	
0.5 —		0.00	TOPSOIL.	
0.5 -			Dry, reddish brown, medium dense to dense, intact, silty sand.  TRANSPORTED.	
1.5 —				
_		1.60	End of	log

NOTES: 1: No water

2: Side walls stable

3: Sample taken at 0.5-1m). (FI, chemical)

4: Easy to medium excavation

5: Refusal at 1.6m

CONTRACTOR: Labante 5 (PTY) Ltd MACHINE: CAT 48.8E TLB PIT LxB: 2x 0.8 m FILE REF: ESKOM.GPJ DATE EXCAVATED: 2015/11/06 DATE PROFILED: 2015/11/06 PROFILED BY: KM/DM CHECKED BY:





CLIENT: ESKOM

PROJECT: Medupi Geotechnical investigation

LOCATION: Lephalale, Limpopo

PROJECT NO: 1415881

HOLE No: MTP6

X COORD: 27.53 Y COORD: -23.72 DATUM: WGS84

ELEVATION:

Page 1 of 1

Scale	Legend	Depth	Description
-		0.00	Dry, brown, loose, intact, fine sand.  TOPSOIL.
0.5 — 1.0 —		_0.40	Dry, reddish brown, medium dense to dense, intact, coarse sand.  TRANSPORTED.
1.5		_1.50	Dry, yellowish brown, <u>dense</u> , homogeneous, cemented gravel excavated as gravelly sand.  TRANSPORTED (PEBBLE MARKER?).
2.5	00000	2.50	End of log

NOTES: 1: No water

Side walls stable
 No sample taken
 Medium excavation
 Hole terminated at 2.5m

CONTRACTOR: Labante 5 (PTY) Ltd

MACHINE: CAT 48.8E TLB

PIT LxB: 2x 0.8 m

FILE REF: ESKOM.GPJ

DATE EXCAVATED: 2015/11/06 DATE PROFILED: 2015/11/06 PROFILED BY: KM/DM CHECKED BY:





CLIENT: ESKOM

PROJECT: Medupi Geotechnical investigation

LOCATION: Lephalale, Limpopo

PROJECT NO: 1415881

HOLE No: MTP8

X COORD: 27.54 Y COORD: -23.71 DATUM: WGS84

ELEVATION:

Page 1 of 1

Scale	Legend	Depth	Description
-		0.00	Dry, brown, loose, intact, fine sand.  TOPSOIL.
0.5 —		_1.00	Dry, reddish brown, medium dense, intact, fine sand.  TRANSPORTED.
		1.50	Dry, reddish brown, dense to very dense, homogeneous, ferruginised fine gravel.  TRANSPORTED.
-			End of log

NOTES: 1: No water

2: Side walls stable3: Medium excavation4: Refusal at 1.5m

5: Sample taken at 0.5m- 1m

CONTRACTOR: Labante 5 (PTY) Ltd
MACHINE: CAT 48.8E TLB
PIT LxB: 2x 0.8 m
FILE REF: ESKOM.GPJ

DATE EXCAVATED: 2015/11/06 DATE PROFILED: 2015/11/06 PROFILED BY: KM/DM CHECKED BY:





CLIENT: ESKOM

PROJECT: Medupi Geotechnical investigation

LOCATION: Lephalale, Limpopo

PROJECT NO: 1415881

HOLE No: MTP9

X COORD: 27.54 Y COORD: -23.70 DATUM: WGS84

ELEVATION:

Page 1 of 1

Scale	Legend	Depth	Description
-		0.00	Dry, brown, loose, intact, fine sand.  TOPSOIL.
0.5 —		0.40	Dry, reddish brown, medium dense, intact, medium-coarse sand and gravel.  TRANSPORTED.
1.5 -		1.40	Reddish dark brown, highly weathered, fine grained, rounded-subangular rock excavated as gravel and cobbles, soft rock
-			End of log

NOTES: 1: No water

2: Side walls stable3: No sample taken4: Hard excavatiom

CONTRACTOR: Labante 5 (PTY) Ltd MACHINE: CAT 48.8E TLB PIT LxB: 2x 0.8 m

FILE REF: ESKOM.GPJ

DATE EXCAVATED: 2015/11/06 DATE PROFILED: 2015/11/06 PROFILED BY: KM/DM CHECKED BY:





CLIENT: ESKOM

PROJECT: Medupi Geotechnical investigation

LOCATION: Lephalale, Limpopo

PROJECT NO: 1415881

HOLE No: MTP10

X COORD: 27.49 Y COORD: -23.75 DATUM: WGS84

ELEVATION:

Page 1 of 1

Scale	Legend	Depth	Description	
0.5 -		0.00	TOPSOIL.	
				End of log

NOTES: 1: No water

2: Side walls stable

3: 2 samples taken at 0.3-1.0m

4: Refusal on dense sand at 1m

CONTRACTOR: Labante 5 (PTY) Ltd

MACHINE: CAT 48.8E TLB

PIT LxB: 2x 0.8 m

FILE REF: ESKOM.GPJ

DATE EXCAVATED: 2015/11/04

DATE PROFILED: 2015/11/04

PROFILED BY: KM/DM

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CLIENT: ESKOM

PROJECT: Medupi Geotechnical investigation

LOCATION: Lephalale, Limpopo

PROJECT NO: 1415881

**HOLE No:** MTP11

X COORD: 27.50 Y COORD: - 23.75 DATUM: WGS84

ELEVATION:

Page 1 of 1

Scale	Legend	Depth	Description
-		0.00	Dry, brown, loose, intact, fine sand with roots.  TOPSOIL.
0.5 -		0.30	Dry, light brown, medium dense to dense, intact, fine-medium coarse sand.  TRANSPORTED.
1.0		1.00	Dry, brown mottled white, dense to very dense, homogeneous, calcretised gravels excavated as sub-rouded to angular, medium to coarse gravels.  TRANSPORTED.
			End of log

NOTES: 1: No water

2: Side walls stable 3: No sample taken

4: Refusal at 1m

5: Medium to hard excavation

CONTRACTOR: Labante 5 (PTY) Ltd MACHINE: CAT 48.8E TLB PIT LxB: 2x 0.8 m FILE REF: ESKOM.GPJ

DATE EXCAVATED: 2015/11/04 DATE PROFILED: 2015/11/04 PROFILED BY: KM/DM CHECKED BY:





CLIENT: ESKOM

PROJECT: Medupi Geotechnical investigation

LOCATION: Lephalale, Limpopo

PROJECT NO: 1415881

HOLE No: MTP12

X COORD: 27.51 Y COORD: -23.75 DATUM: WGS84

ELEVATION:

Page 1 of 1

Scale	Legend	Depth	Description
-		0.00	Dry, brown, <u>loose to medium dense</u> , intact, <u>fine sand</u> with roots.  TOPSOIL.
0.5 —		_0.40	Dry, reddish brown, loose to medium dense, intact, fine sand.  TRANSPORTED.
1.0 —		_1.20	Dry, reddish brown, <u>medium dense to dense</u> , homogeneous, cemented gravel.  TRANSPORTED.
1.5 —			
_		2.40	
2.5 —			End of log

NOTES: 1: No water

2: Side walls stable

3: Sample taken at 1.4m, (FI)4: Excavation stopped at 2.4m

5: Medium excavation

CONTRACTOR: Labante 5 (PTY) Ltd

MACHINE: CAT 48.8E TLB

PIT LxB: 2x 0.8 m

FILE REF: ESKOM.GPJ

DATE EXCAVATED: 2015/11/05 DATE PROFILED: 2015/11/05 PROFILED BY: KM/DM CHECKED BY:





CLIENT: ESKOM

PROJECT: Medupi Geotechnical investigation

LOCATION: Lephalale, Limpopo

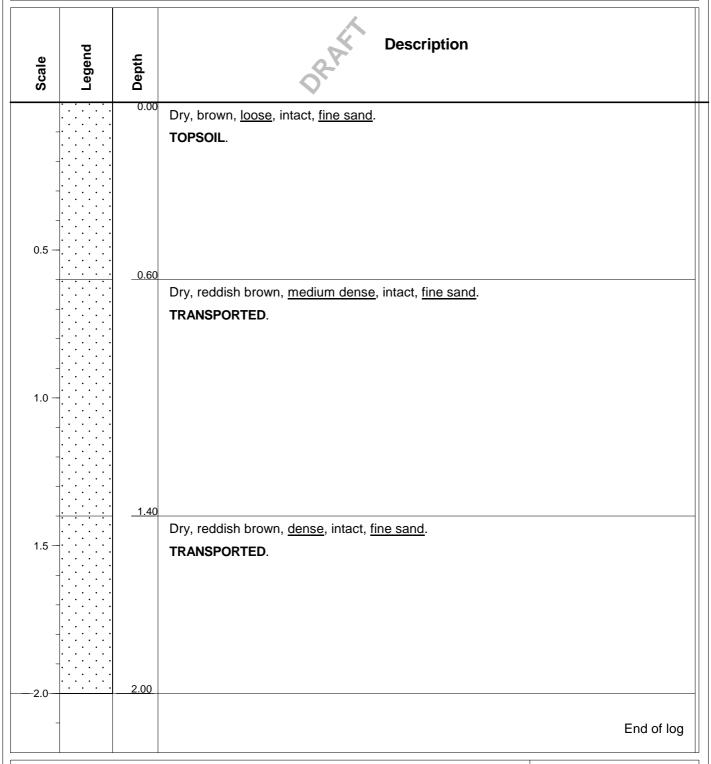
PROJECT NO: 1415881

HOLE No: MTP13

X COORD: 27.50 Y COORD: -23.74 DATUM: WGS84

ELEVATION:

Page 1 of 1



NOTES: 1: No water

2: Side walls stable3: No sample taken

4: Refusal on dense sand at 2m

CONTRACTOR: Labante 5 (PTY) Ltd

MACHINE: CAT 48.8E TLB

PIT LxB: 2x 0.8 m

FILE REF: ESKOM.GPJ

DATE EXCAVATED: 2015/11/04 DATE PROFILED: 2015/11/04 PROFILED BY: KM/DM CHECKED BY:





CLIENT: ESKOM

PROJECT: Medupi Geotechnical investigation

LOCATION: Lephalale, Limpopo

PROJECT NO: 1415881

HOLE No: MTP14

X COORD: 27.51 Y COORD: -23.74 DATUM: WGS84

ELEVATION:

Page 1 of 1

Scale	Legend	Depth	Description
0.5 -		0.00 _0.20	TOPSOIL.
		1.80	End of log

NOTES: 1: No water

2: Side walls stable

3: Sample taken at 1.5m

4: Refusal cemented gravel at 1.8m

5: Medium to hard excavation

CONTRACTOR: Labante 5 (PTY) Ltd

MACHINE: CAT 48.8E TLB

PIT LxB: 2x 0.8 m

FILE REF: ESKOM.GPJ

DATE EXCAVATED: 2015/11/05 DATE PROFILED: 2015/11/05 PROFILED BY: KM/DM CHECKED BY:





CLIENT: ESKOM

PROJECT: Medupi Geotechnical investigation

LOCATION: Lephalale, Limpopo

PROJECT NO: 1415881

HOLE No: MTP15

X COORD: 27.52 Y COORD: -23.74 DATUM: WGS84

ELEVATION:

Page 1 of 1

Scale	Legend	Depth	Description
-		0.00	TOPSOIL.
0.5 -			Dry, brown, dense, intact, sand.  TRANSPORTED.
-1.0-		1.00	Dry, brown mottled orange, very dense, gravelly sand.  RESIDUAL.
1.0			End of log

NOTES: 1: No water

2: Side walls stable

3: Sample taken at 1m (chemical)4: Refusal on residual material at 1m5: Soft to medium excavation

CONTRACTOR: Labante 5 (PTY) Ltd

MACHINE: CAT 48.8E TLB

PIT LxB: 2x 0.8 m

FILE REF: ESKOM.GPJ

DATE EXCAVATED: 2015/11/05 DATE PROFILED: 2015/11/05 PROFILED BY: KM/DM CHECKED BY:





CLIENT: ESKOM

PROJECT: Medupi Geotechnical investigation

LOCATION: Lephalale, Limpopo

PROJECT NO: 1415881

HOLE No: MTP16

X COORD: 27.53 Y COORD: -23.74 DATUM: WGS84

ELEVATION:

Page 1 of 1

Scale	Legend	Depth	Description	
0.5 -		0.40	TOPSOIL.	
_			End	of log

NOTES: 1: No water

2: Side walls stable

3: Sample taken at 0.4-1.2m

4: Medium to hard excavation

5: Refusal at 1.2m

CONTRACTOR: Labante 5 (PTY) Ltd MACHINE: CAT 48.8E TLB PIT LxB: 2x 0.8 m

FILE REF: ESKOM.GPJ

DATE EXCAVATED: 2015/11/05 DATE PROFILED: 2015/11/05 PROFILED BY: KM/DM CHECKED BY:





CLIENT: ESKOM

PROJECT: Medupi Geotechnical investigation

LOCATION: Lephalale, Limpopo

PROJECT NO: 1415881

HOLE No: MTP17

X COORD: 27.51 Y COORD: -23.73 DATUM: WGS84

ELEVATION:

Page 1 of 1

Scale	Legend	Depth	Description	
0.5 —		0.50	TOPSOIL.	
1.0-		1.00		
			I	End of log

NOTES: 1: No water

2: Side walls stable 3: Refusal at 1m

4: 2 samples taken at 1m (FI,FH permeability)

5: Medium to hard excavation

CONTRACTOR: Labante 5 (PTY) Ltd

MACHINE: CAT 48.8E TLB

PIT LxB: 2x 0.8 m

FILE REF: ESKOM.GPJ

DATE EXCAVATED: 2015/11/05 DATE PROFILED: 2015/11/05 PROFILED BY: KM/DM CHECKED BY:





CLIENT: ESKOM

PROJECT: Medupi Geotechnical investigation

LOCATION: Lephalale, Limpopo

PROJECT NO: 1415881

HOLE No: MTP19

X COORD: 27.53 Y COORD: -23.74 DATUM: WGS84

ELEVATION:

Page 1 of 1

Scale	Legend	Depth	Description	
-		0.00	RESIDUAL.	
0.5 -		0.30	Dry, brown, medium dense to dense, intact, fine sand.  TRANSPORTED.	
1.0 -		<u>0.90</u>	Dry, brown speckled red, dense, homogeneous, gravelly sand.  RESIDUAL.	
				End of log

NOTES: 1: No water

2: Side walls stable3: Sample at 0.9-1.2m4: Easy to medium excavation

5: Refusal at 1.2m

CONTRACTOR: Labante 5 (PTY) Ltd MACHINE: CAT 48.8E TLB PIT LxB: 2x 0.8 m FILE REF: ESKOM.GPJ DATE EXCAVATED: 2015/11/05 DATE PROFILED: 2015/11/05 PROFILED BY: KM/DM CHECKED BY:





CLIENT: ESKOM

PROJECT: Medupi Geotechnical investigation

LOCATION: Lephalale, Limpopo

PROJECT NO: 1415881

HOLE No: MTP20

X COORD: 27.53 Y COORD: -23.73 DATUM: WGS84

ELEVATION:

Page 1 of 1

Scale	Legend	Depth	Description	
-		0.00	TOPSOIL.	
0.5 —		_0.30	Dry, brown, dense to very dense, intact, coarse sand.  TRANSPORTED.	
-		_1.20	Dry, whittish brown, <u>very dense</u> , homogeneous, calcretised gravel.  TRANSPORTED (PEDOGENIC?).	
-			E	End of log

NOTES: 1: No water

2: Side walls stable3: No sample taken

4: Easy to medium excavation

5: Refusal at 1.5m

CONTRACTOR: Labante 5 (PTY) Ltd MACHINE: CAT 48.8E TLB PIT LxB: 2x 0.8 m FILE REF: ESKOM.GPJ DATE EXCAVATED: 2015/11/05 DATE PROFILED: 2015/11/05 PROFILED BY: KM/DM CHECKED BY:





Project: Medupi Hydrogeological Study

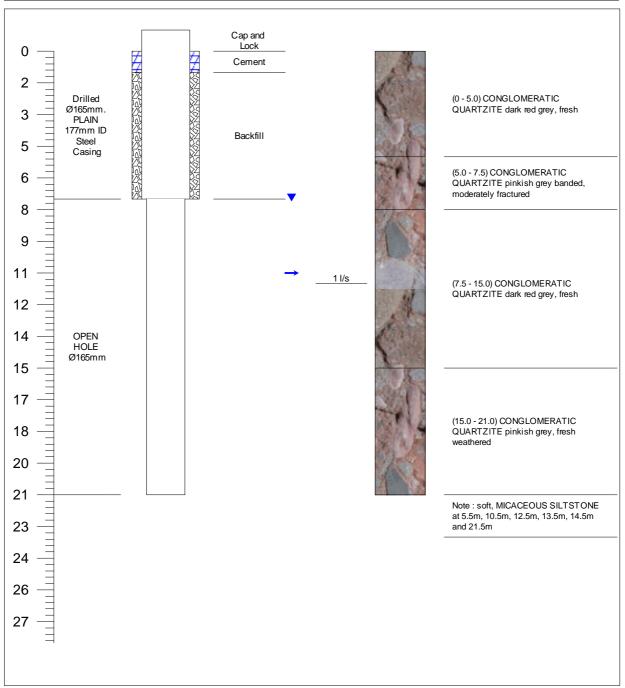
Location: Medupi Power Station

Latitude: -57148 S Project No.:12087

Longitude: 2622479 E Borehole Number

Elevation: 900.81 mamsl GA001

Depth Well Water strike(s)
(m) Construction Water level Lithology Lithology Description
(mbgl)



Date drilled: Drilling contractor: Hydrogeologist 18/02/2009 Brewis J. Pretorius Drilling diameter: 165 mm Final blow yield: 1 l/s Collar height: -1.28 m





Project: Medupi Hydrogeological Study

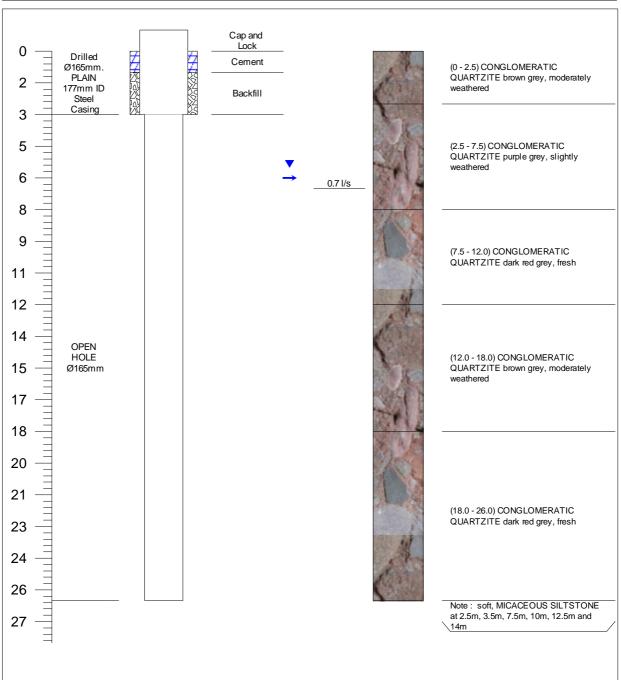
Location: Medupi Power Station

Latitude: -57183 S Project No.:12087

Longitude: 2622654 E Borehole Number

Elevation: 901.54 mamsl GA002

Depth Well Water strike(s)
(m) Construction Water level Lithology Lithology Description (mbgl)



Date drilled: Drilling contractor: Hydrogeologist 23/02/2009 Brewis J. Pretorius Drilling diameter: 165 mm Final blow yield: 0.7 l/s Collar height: -1.34m





Project: Medupi Hydrogeological Study

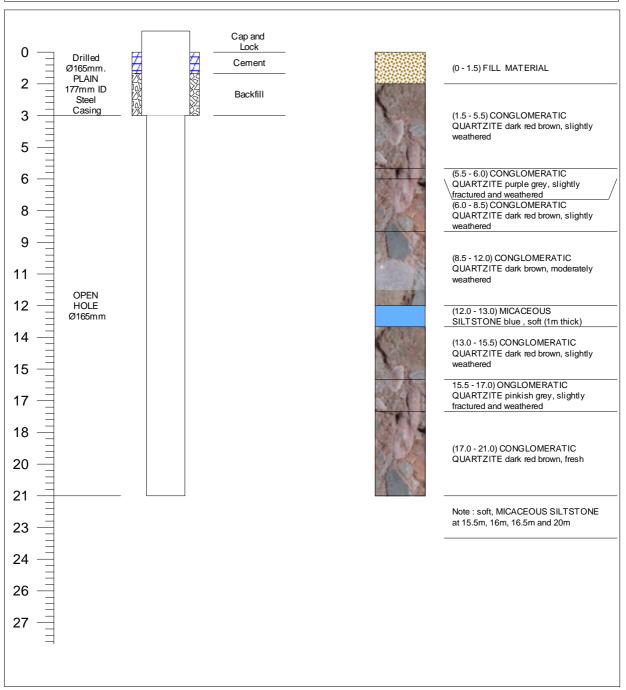
Location: Medupi Power Station

Latitude: -57307 S Project No.:12087

Longitude: 2622868 E Borehole Number

Elevation: 902.77 mamsl GA003

Depth Well Water strike(s)
(m) Construction Water level Lithology Lithology Description
(mbgl)



Date drilled: Drilling contractor: Hydrogeologist 23/02/2009 Brewis J. Pretorius Drilling diameter: 165 mm Final blow yield: DRY Collar height: -1.36m





Project: Medupi Hydrogeological Study

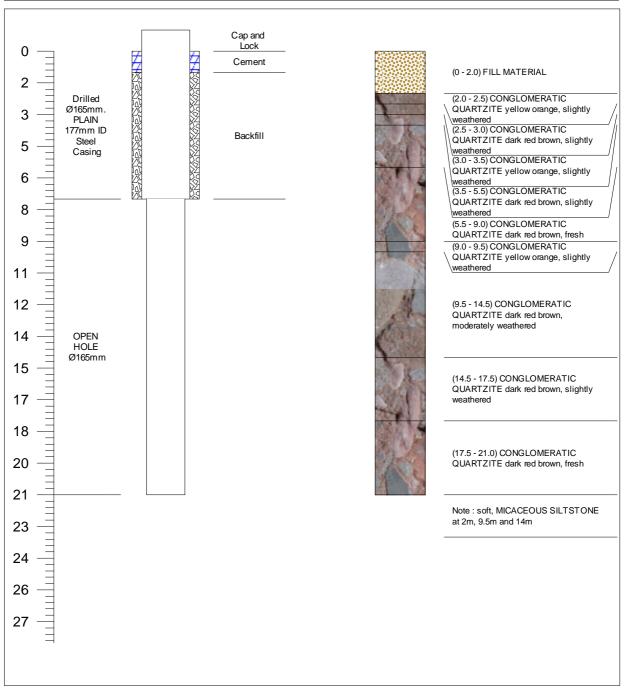
Location: Medupi Power Station

Latitude: -57410 S Project No.:12087

Longitude: 2623058 E Borehole Number

Elevation: 902.34 mamsl GA004

Depth Well Water strike(s)
(m) Construction Water level Lithology Lithology Description (mbgl)



Date drilled: Drilling contractor: Hydrogeologist 17/022009 Brewis J. Pretorius Drilling diameter: 165 mm Final blow yield: DRY Collar height: -0.66 m





Project: Medupi Hydrogeological Study

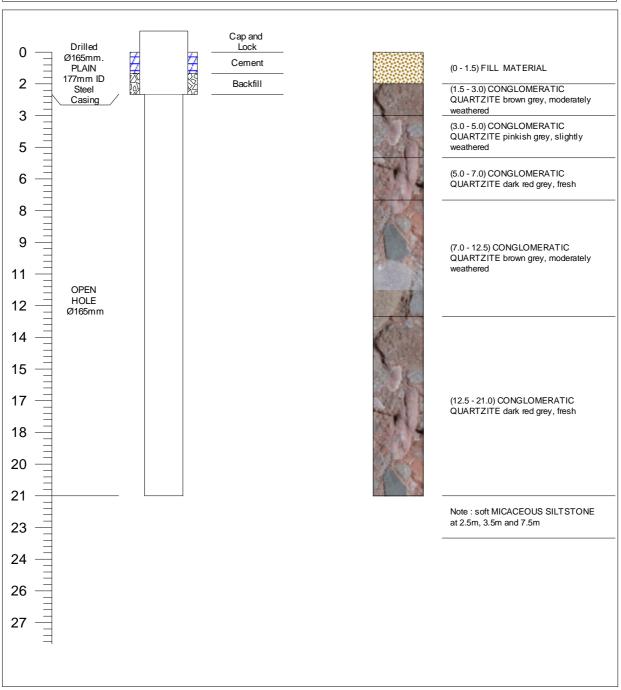
Location: Medupi Power Station

Latitude: -57555 S Project No.:12087

Longitude: 2623117 E Borehole Number

Elevation: 901.91 mamsl GA005

Depth Well Water strike(s)
(m) Construction Water level Lithology Lithology Description (mbgl)



Date drilled: Drilling contractor: Hydrogeologist 16/02/2009 Brewis J. Pretorius Drilling diameter: 165 mm Final blow yield: DRY Collar height: -1.37m





Project: Medupi Hydrogeological Study

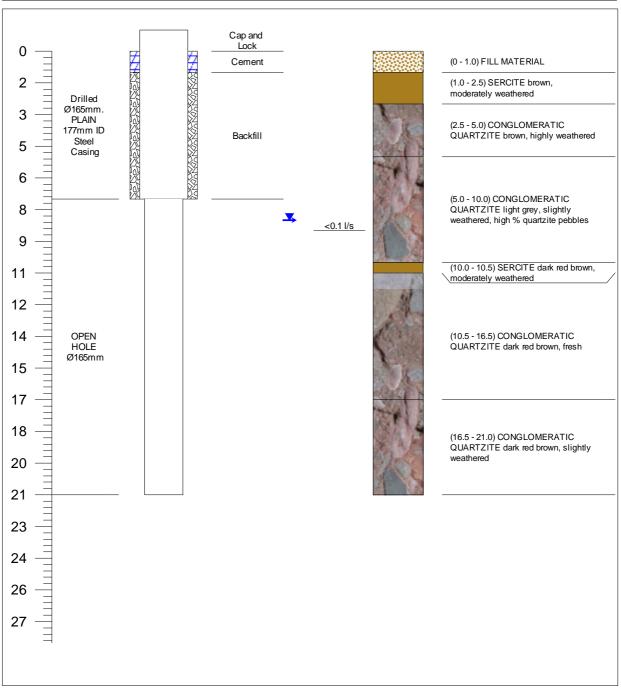
Location: Medupi Power Station

Latitude: -57380 S Project No.:12087

Longitude: 2622283 E Borehole Number

Elevation: 900.75 mamsl GA006

Depth Well Water strike(s)
(m) Construction Water level Lithology Lithology Description (mbgl)



Date drilled: Drilling contractor: Hydrogeologist 21/02/2009 Brewis J. Pretorius Drilling diameter: 165 mm Final blow yield: <0.1 l/s Collar height: -1.45 m





Project: Medupi Hydrogeological Study

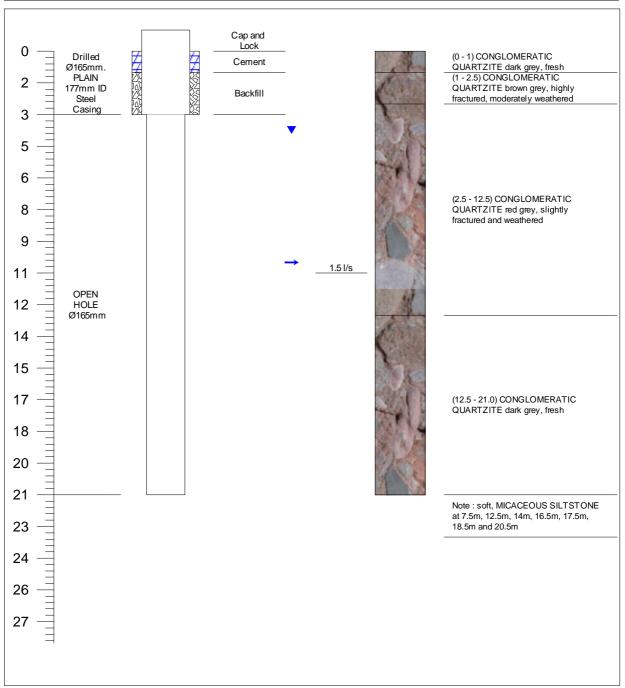
Location: Medupi Power Station

Latitude: -57367 S Project No.:12087

Longitude: 2622416 E Borehole Number

Elevation: 896.62 mamsl GA007

Depth Well Water strike(s)
(m) Construction Water level Lithology Lithology Description
(mbgl)



Date drilled: Drilling contractor: Hydrogeologist 20/02/2009 Brewis J. Pretorius Drilling diameter: 165 mm Final blow yield: 1.5 l/s Collar height: -1.47m





Project: Medupi Hydrogeological Study

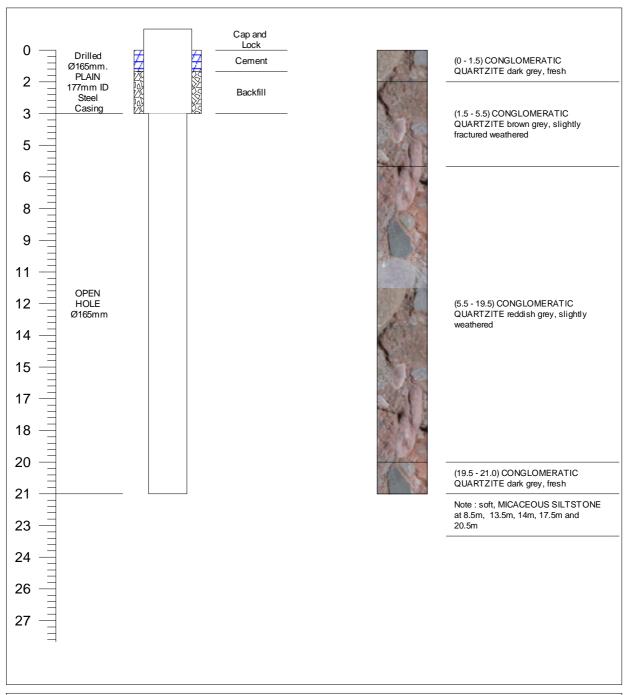
Location: Medupi Power Station

Latitude: -57521 S Project No.:12087

Longitude: 2622418 E Borehole Number

Elevation: 896.23 mamsl GA008

Depth Well Water strike(s)
(m) Construction (mbgl) Lithology Lithology Description



Date drilled: Drilling contractor: Hydrogeologist 20/02/2009 Brewis J. Pretorius Drilling diameter: 165 mm Final blow yield: DRY Collar height: -1.15 m





Project: Medupi Hydrogeological Study

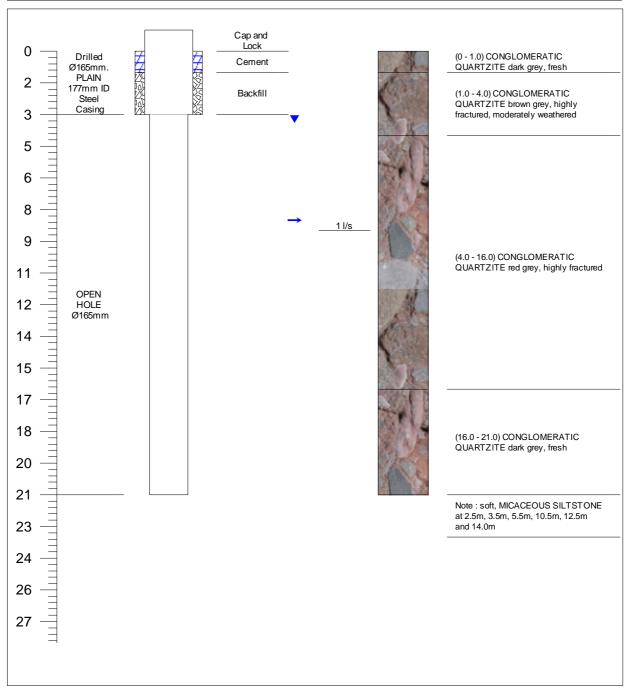
Location: Medupi Power Station

Latitude: -57356 S Project No.:12087

Longitude: 2622604 E Borehole Number

Elevation: 895.23 mamsl GA009

Depth Well Water strike(s)
(m) Construction Water level Lithology Lithology Description
(mbgl)



Date drilled: Drilling contractor: Hydrogeologist 23/02/2009 Brewis J. Pretorius Drilling diameter: 165 mm Final blow yield: 1 l/s Collar height: -1.29 m





Project: Medupi Hydrogeological Study

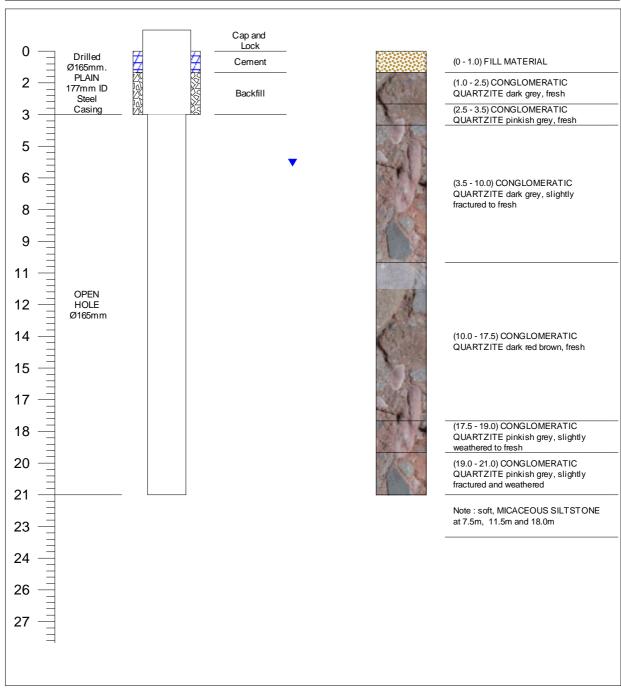
Location: Medupi Power Station

Latitude: -57643 S Project No.:12087

Longitude: 2622808 E Borehole Number

Elevation: 898.13 mamsl GA010

Depth Well Water strike(s)
(m) Construction Water level Lithology Lithology Description
(mbgl)



Date drilled: Drilling contractor: Hydrogeologist 17/02/2009 Brewis J. Pretorius Drilling diameter: 165 mm Final blow yield: DRY Collar height: -1.23 m





Project: Medupi Hydrogeological Study

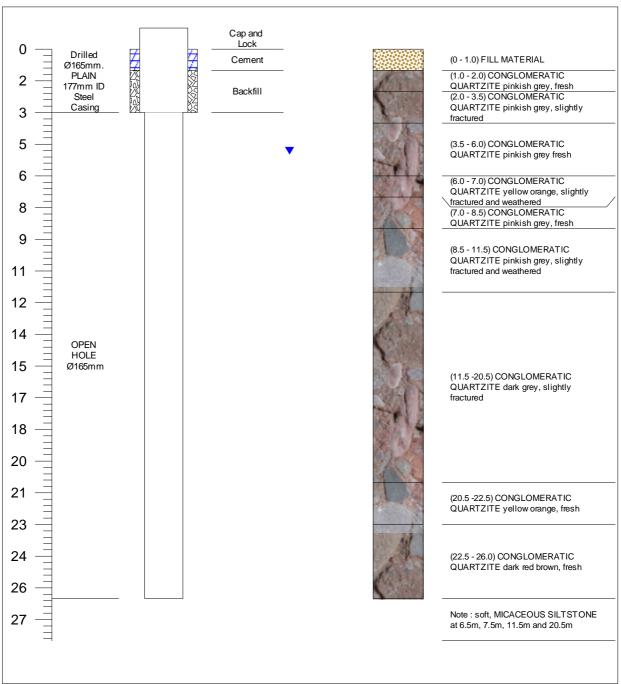
Location: Medupi Power Station

Latitude: -57769 S Project No.:12087

Longitude: 2623026 E Borehole Number

Elevation: 899 mamsl GA011

Depth Well Water strike(s)
(m) Construction Water level Lithology Lithology Description (mbgl)



Date drilled: Drilling contractor: Hydrogeologist 18/02/2009 Brewis J. Pretorius Drilling diameter: 165 mm Final blow yield: 0.1l/s Collar height: -1.25 m





Project: Medupi Hydrogeological Study

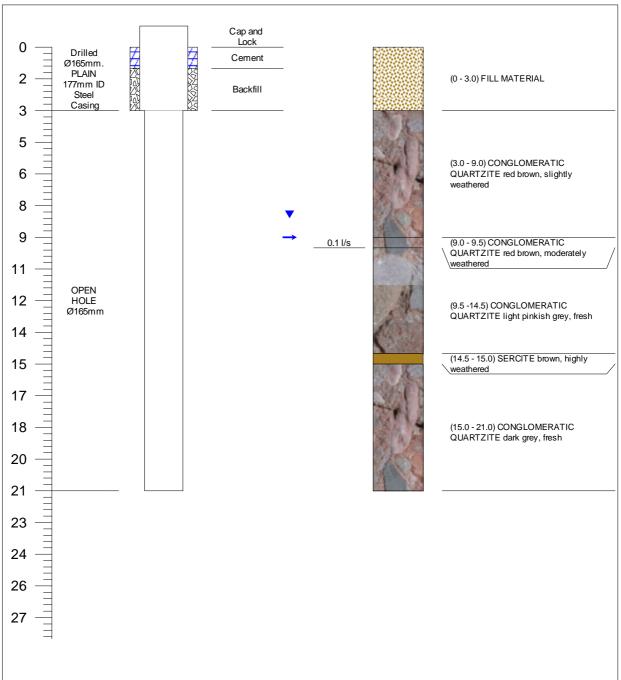
Location: Medupi Power Station

Latitude: -57545 S Project No.:12087

Longitude: 2622271 E Borehole Number

Elevation: 900.15 mamsl GA012

Depth Well Water strike(s)
(m) Construction Water level Lithology Lithology Description (mbgl)



Date drilled: Drilling contractor: Hydrogeologist 20/02/2009 Brewis J. Pretorius Drilling diameter: 165 mm Final blow yield: 0.1 l/s Collar height: -1.19m





Project: Medupi Hydrogeological Study

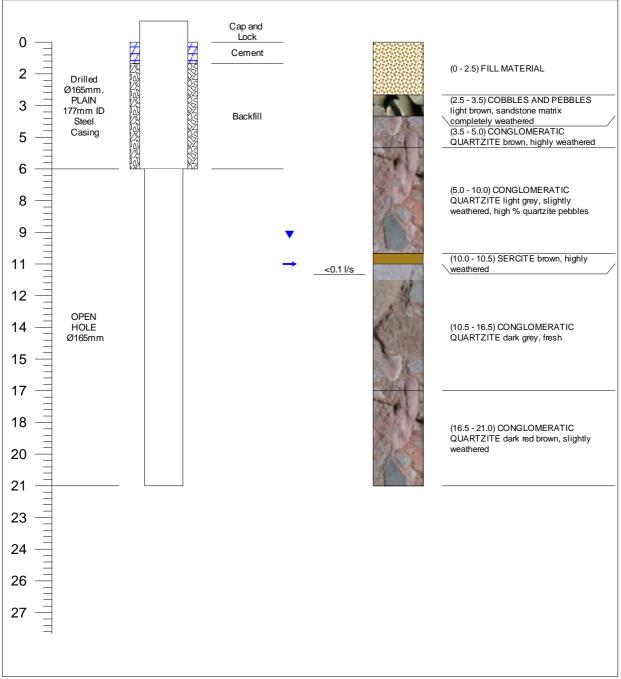
Location: Medupi Power Station

Latitude: -57659 S Project No.:12087

Longitude: 2622448 E Borehole Number

Elevation: 901.6 mamsl GA014

Depth Well Water strike(s) and Water level Lithology Lithology Description (mbgl)



Date drilled: Drilling contractor: Hydrogeologist 23/02/2009 Brewis J. Pretorius Drilling diameter: 165 mm Final blow yield: <0.1 l/s Collar height: -1.36 m





Project: Medupi Hydrogeological Study

Location: Medupi Power Station

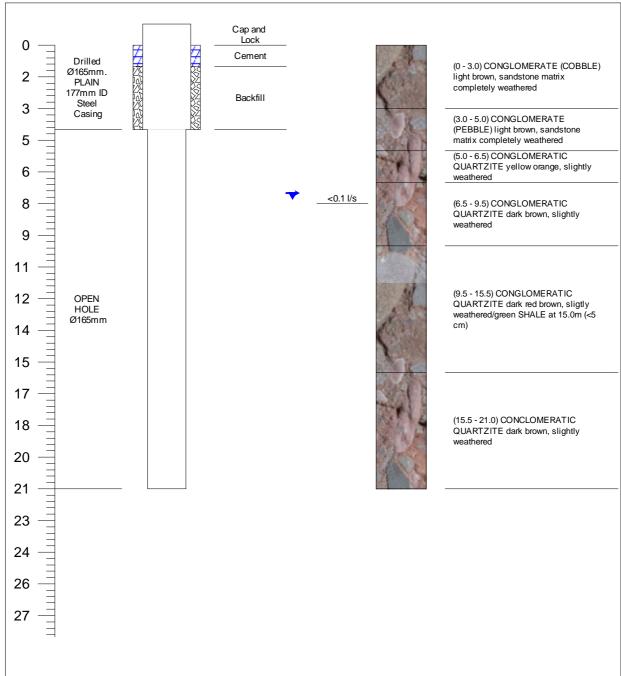
Latitude: -57644 S Project No.:12087

Longitude: 2622547 E Borehole Number

Elevation: 899.25 mamsl GA015

Depth Well Water strike(s) and Water level (mbgl) Lithology Lithology Description

Cap and



Date drilled: Drilling contractor: Hydrogeologist 20/02/2009 Brewis J. Pretorius Drilling diameter: 165 mm Final blow yield: 0.1l/s Collar height: -1.38 m





Project: Medupi Hydrogeological Study

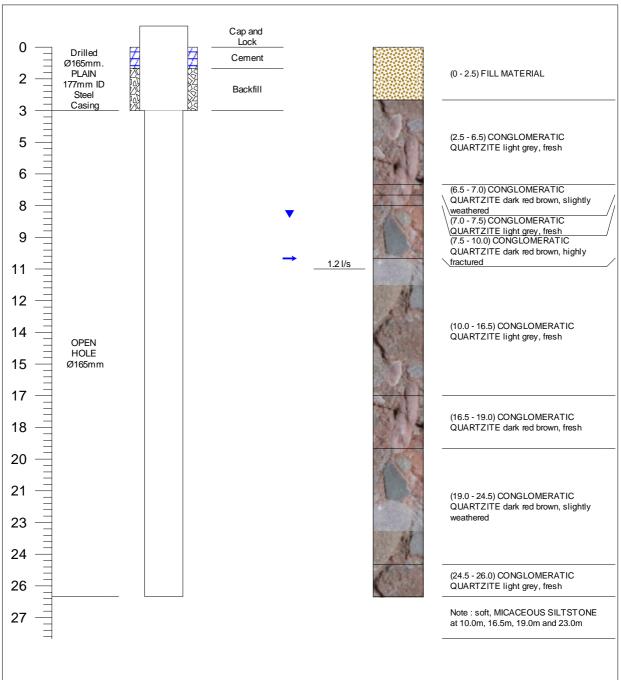
Location: Medupi Power Station

Latitude: -57786 S Project No.:12087

Longitude: 2622683 E Borehole Number

Elevation: 901.38 mamsl GA016

Depth Well Water strike(s) and Water level Lithology Lithology Description (mbgl)



Date drilled: Drilling contractor: Hydrogeologist 17/02/2009 Brewis J. Pretorius Drilling diameter: 165 mm Final blow yield: 1.2 l/s Collar height: -1.22m





Project: Medupi Hydrogeological Study

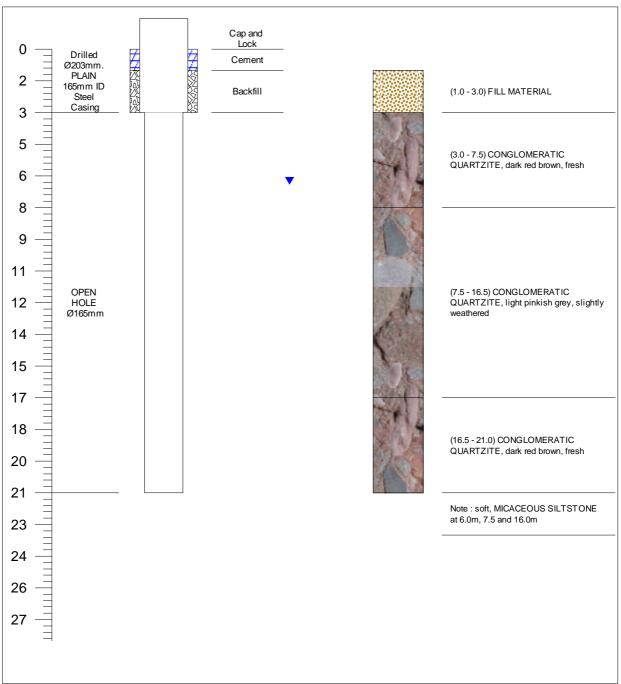
Location: Medupi Power Station

Latitude: -57899 S Project No.:12087

Longitude: 2622866 E Borehole Number

Elevation: 900.68 mamsl GA017

Depth Well Water strike(s)
(m) Construction Water level Lithology Lithology Description (mbgl)



Date drilled: Drilling contractor: Hydrogeologist 17/02/2009 Brewis J. Pretorius Drilling diameter: 165 mm Final blow yield: DRY Collar height: -1.45 m





Project: Medupi Hydrogeological Study

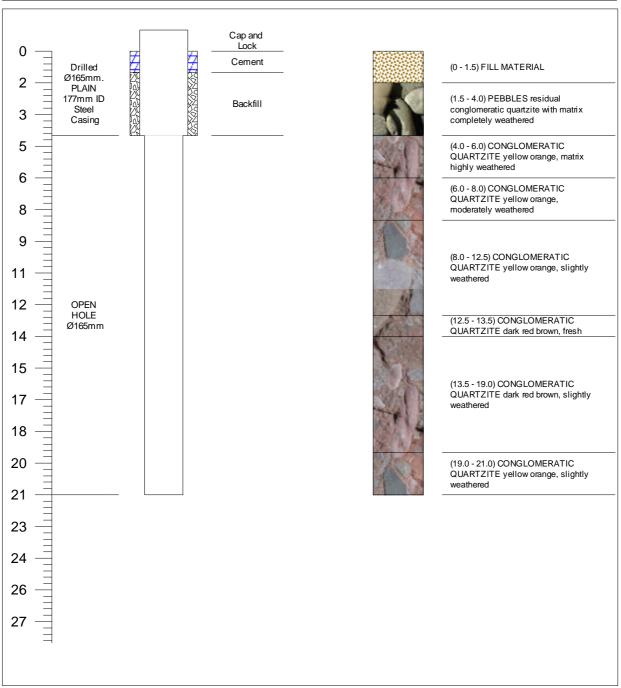
Location: Medupi Power Station

Latitude: -56948 S Project No.:12087

Longitude: 2622332 E Borehole Number

Elevation: 902.11 mamsl GA018

Depth Well Water strike(s)
(m) Construction Water level Lithology Lithology Description (mbgl)



Date drilled: Drilling contractor: Hydrogeologist 23/02/2009 Brewis J. Pretorius Drilling diameter: 165 mm Final blow yield: DRY Collar height: -1.2 m





Project: Medupi Hydrogeological Study

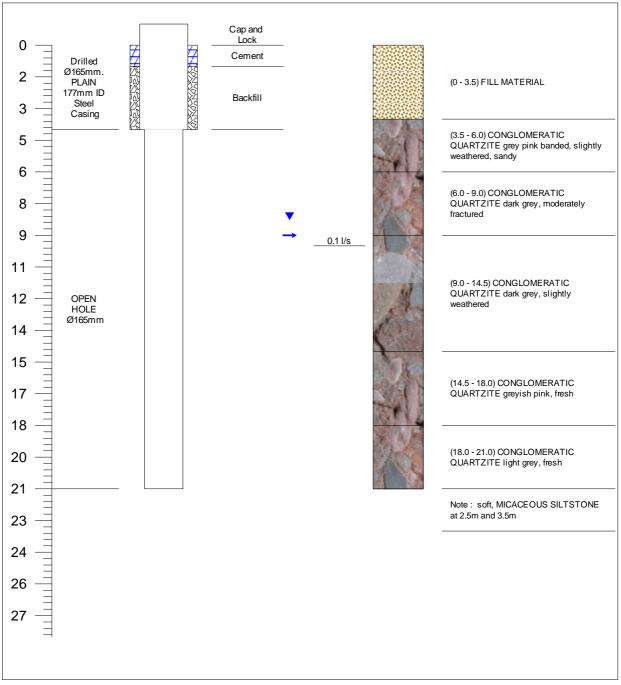
Location: Medupi Power Station

Latitude: -56960 S Project No.:12087

Longitude: 2622601 E Borehole Number

Elevation: 903.92 mamsl GA019

Depth Well Water strike(s) and Water level Lithology Lithology Description (mbgl)



Date drilled: Drilling contractor: Hydrogeologist 19/02/2009 Brewis J. Pretorius Drilling diameter: 165 mm Final blow yield: 0.1 l/s Collar height: -0.74m





Project: Medupi Hydrogeological Study

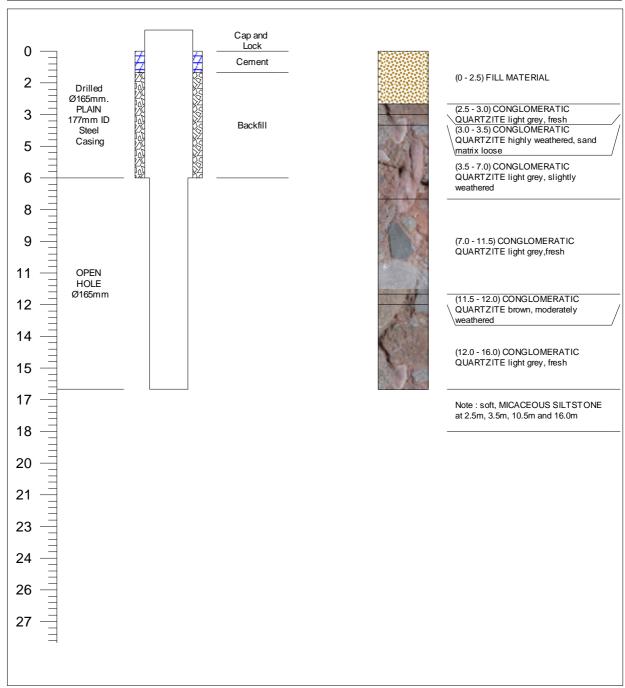
Location: Medupi Power Station

Latitude: -57203 S Project No.:12087

Longitude: 2623036 E Borehole Number

Elevation: 904.66 mamsl GA020D

Depth Well Water strike(s)
(m) Construction Water level Lithology Lithology Description (mbgl)



Date drilled: Drilling contractor: Hydrogeologist 16/02/2009 Brewis J. Pretorius Drilling diameter: 165 mm Final blow yield: DRY Collar height: -1.13m





Project: Medupi Hydrogeological Study

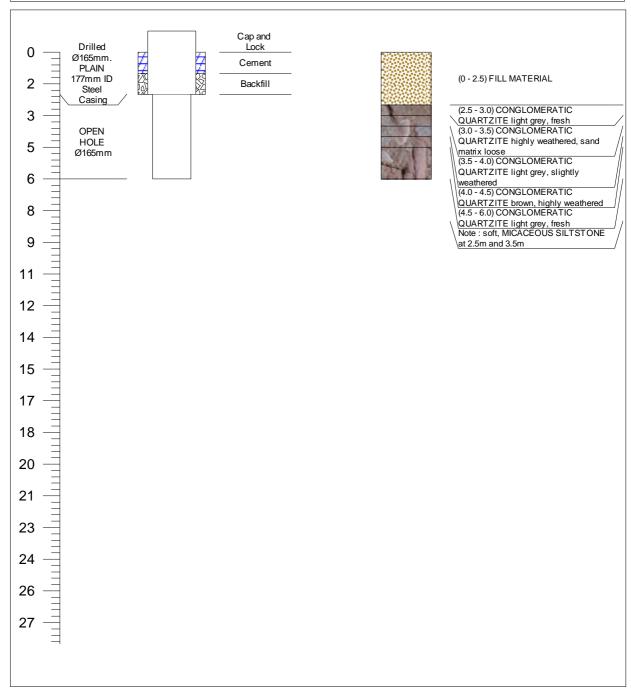
Location: Medupi Power Station

Latitude: -57203 S Project No.:12087

Longitude: 2623036 E Borehole Number

Elevation: 904.66 mamsl GA020S

Depth Well Water strike(s)
(m) Construction (mbgl) Lithology Lithology Description



Date drilled: Drilling contractor: Hydrogeologist 16/02/2009 Brewis J. Pretorius Drilling diameter: 165 mm Final blow yield: DRY Collar height: -1.37m





Project: Medupi Hydrogeological Study

Location: Medupi Power Station

Latitude: -57366 S Project No.:12087

Longitude: 2623259 E Borehole Number

Elevation: 903.2 mamsl GA021

Water strike(s) Depth (m) Well Lithology Description and Water level Lithology Construction (mbgl) Cap and Lock Cement Drilled Ø165mm. (0 - 4.0) FILL MATERIAL PLAIN 177mm ID 3 Backfill Steel (4.0 - 5.5) CONGLOMERATIC 5 QUARTZITE light brown, slightly to medium weathered (5.5 - 6.0) CONGLOMERATIC 6 QUARTZITE dark red brown, slightly weathered (6.0 - 7.0) CONGLOMERATIC QUARTZITE dark grey, slightly 8 fractured to fresh (7.0 - 7.5) CONGLOMERATIC 9 QUARTZITE dark brown, weathered to clay, (Damp) (7.5 - 11.0) CONGLOMERATIC 11 QUARTZITE dark red brown, slightly weathered (11.0 - 11.5) CONGLOMERATATIC 12 QUARTZITE yellow orange, medium **OPEN** weathered, Sercite forming HOLE (11.5 - 14.5) CONGLOMERATIC Ø165mm QUARTZITE dark brown, slightly weathered 15 17 (14.5 - 21.0) CONGLOMERATIC QUARTZITE dark red brown, slightly 18 weathered 20 21 Note: soft, MICACEOUS SILTSTONE at 13.5m 23 24 26 27

Date drilled: Drilling contractor: Hydrogeologist 19/02/2009 Brewis J. Pretorius Drilling diameter: 165 mm Final blow yield: 0.1l/s Collar height: -1.27 m





Project: Medupi Hydrogeological Study

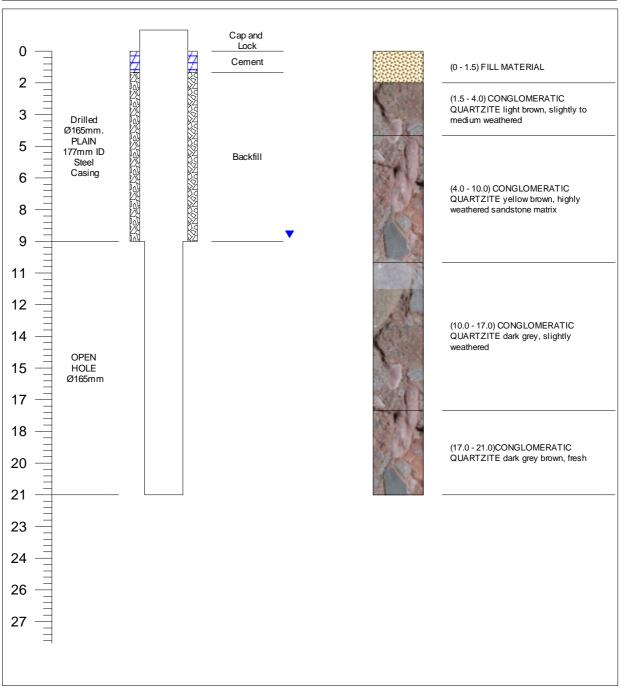
Location: Medupi Power Station

Latitude: -57783 S Project No.:12087

Longitude: 262222 E Borehole Number

Elevation: 899.33 mamsl GA022

Depth Well Water strike(s)
(m) Construction Water level Lithology Lithology Description
(mbgl)



Date drilled: Drilling contractor: Hydrogeologist 18/02/2009 Brewis J. Pretorius Drilling diameter: 165 mm Final blow yield: DRY Collar height: -1.46 m





Project: Medupi Hydrogeological Study

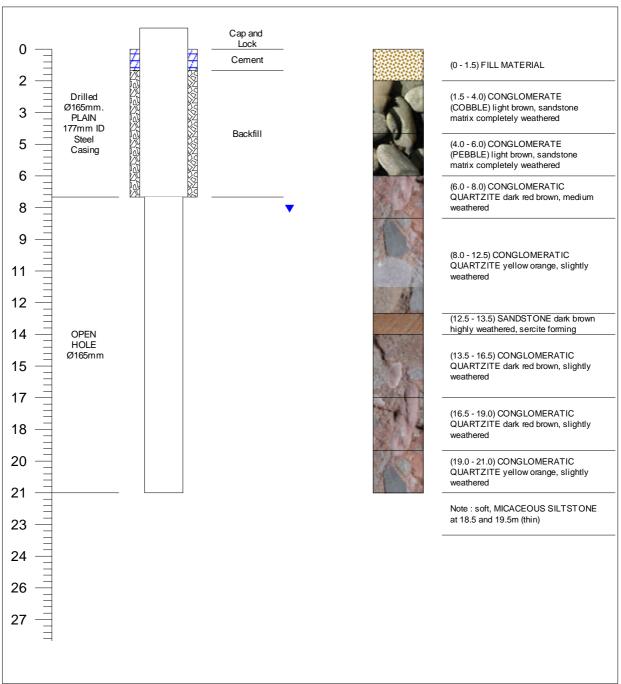
Location: Medupi Power Station

Latitude: -57846 S Project No.:12087

Longitude: 2622415 E Borehole Number

Elevation: 900.36 mamsl GA023

Depth Well Water strike(s)
(m) Construction (mbgl) Lithology Lithology Description



Date drilled: Drilling contractor: Hydrogeologist 20/02/2009 Brewis J. Pretorius Drilling diameter: 165 mm Final blow yield: 0.1l/s Collar height: -1.2 m





Project: Medupi Hydrogeological Study

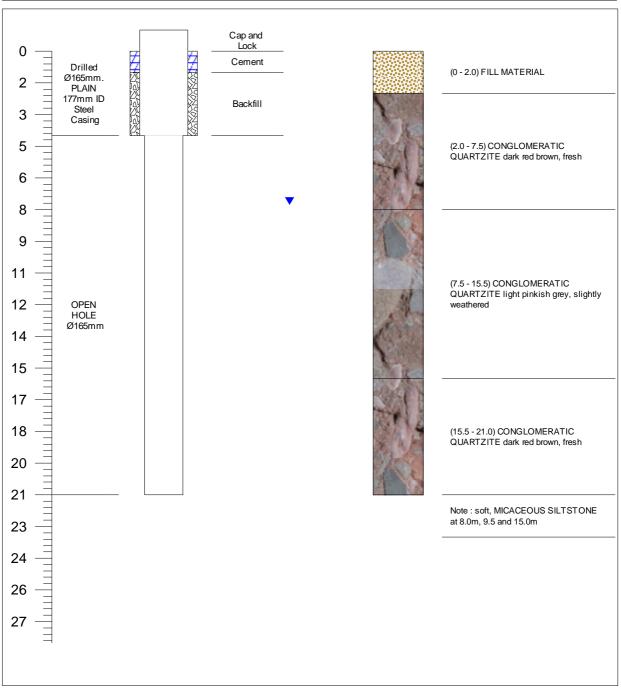
Location: Medupi Power Station

Latitude: -58010 S Project No.:12087

Longitude: 2622649 E Borehole Number

Elevation: 899.24 mamsl GA024

Depth Well Water strike(s) and Water level Lithology Lithology Description (mbgl)



Date drilled: Drilling contractor: Hydrogeologist 19/02/2009 Brewis J. Pretorius Drilling diameter: 165 mm Final blow yield: DRY Collar height: -0.85 m





Project: Medupi Hydrogeological Study

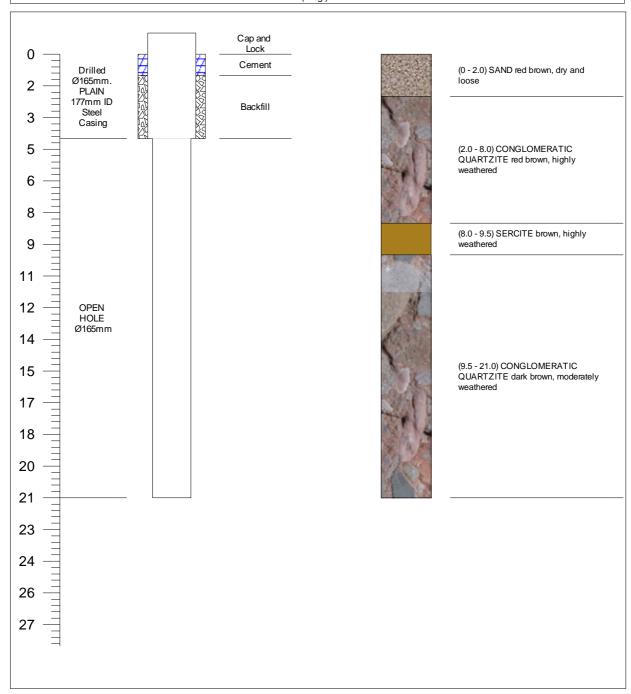
Location: Medupi Power Station

Latitude: -56296 S Project No.:12087

Longitude: 2622495 E Borehole Number

Elevation: 902.39 mamsl GA025

Depth Well Water strike(s)
(m) Construction Water level Lithology Lithology Description
(mbgl)



Date drilled: Drilling contractor: Hydrogeologist 24/02/2009 Brewis J. Pretorius Drilling diameter: 165 mm Final blow yield: DRY Collar height: -1.16m





Project: Medupi Hydrogeological Study

Location: Medupi Power Station

Latitude: -56213 S Project No.:12087

Longitude: 2623131 E Borehole Number

Elevation: 902.04 mamsl GA026

Water strike(s) Depth (m) Well and Water level (mbgl) Lithology Description Lithology Construction Cap and Lock Cement Drilled Ø165mm. (0 - 3.5) SANDY SOIL PLAIN 177mm ID Backfill Steel 3 Casing 5 (3.5 - 6.5) CONGLOMERATIC QUARTZITE grey pink banded, slightly weathered, sandy 6 8 (6.5 - 10.5) CONGLOMERATIC QUARTZITE dark grey, moderately 9 weathered 11 12 OPEN (10.5 - 14.5) CONGLOMERATIC QUARTZITE dark grey, slightly HOLE Ø165mm weathered 14 15 (14.5 - 17.0) CONGLOMERATIC QUARTZITE greyish pink, fresh 17 18 (17.0 - 21.0) CONGLOMERATIC QUARTZITE light grey, fresh 20 21 Note: soft, MICACEOUS SILTSTONE at 2.5m and 3.5m 23 24 26

Date drilled: Drilling contractor: Hydrogeologist

27

24/02/2009 Brewis J. Pretorius Drilling diameter: 165 mm Final blow yield: DRY Collar height: -1.5 m





Project: Medupi Hydrogeological Study

Location: Medupi Power Station

Latitude: -56892 S Project No.:12087

Longitude: 2623224 E Borehole Number

Elevation: 898.82 mamsl GA027

Water strike(s) Depth (m) Well and Water level (mbgl) Lithology Lithology Description Construction Cap and Lock Cement Drilled (0 - 1.5) SANDY SOIL Ø165mm. PLAIN 177mm ID Steel Backfill 3 Casing 5 (1.5 - 9.5) CONGLOMERATIC QUARTZITE grey pink banded, slightly 6 weathered 8 9 (9.5 - 10.5) CONGLOMERATIC QUARTZITE dark grey, moderately 11 fractured OPEN HOLE 12

(16.5 - 21.0) CONGLOMERATIC QUARTZITE greyish pink, fresh

(10.5 - 16.5) CONGLOMERATIC QUARTZITE dark grey, slightly

weathered

Note : soft, MICACEOUS SILTSTONE at 8.5m and 13.5m

Date drilled: Drilling contractor: Hydrogeologist

Ø165mm

14

15

17

18

20

21

23

24

26

27

24/02/2009 Brewis J. Pretorius Drilling diameter: 165 mm Final blow yield: DRY Collar height: -1.12m





Project: Medupi Hydrogeological Study

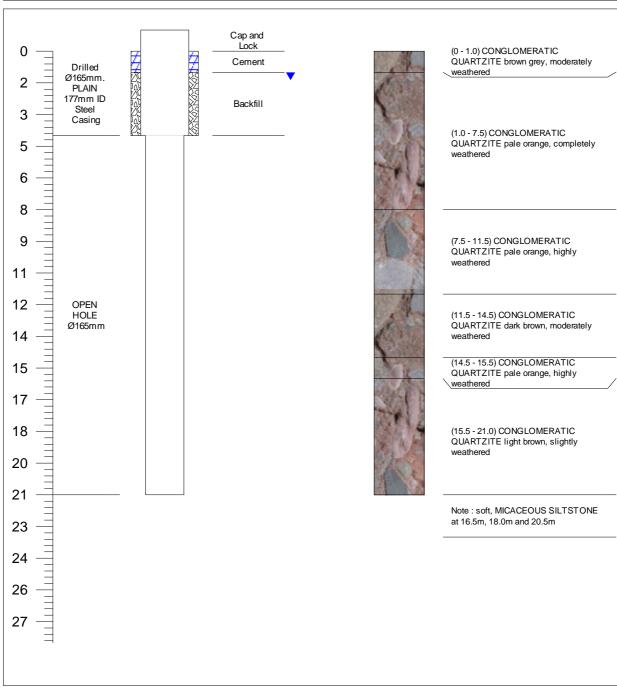
Location: Medupi Power Station

Latitude: -57475 S Project No.:12087

Longitude: 2623619 E Borehole Number

Elevation: 894.18 mamsl GA029

Depth Well Water strike(s) and Water level Lithology Lithology Description (mbgl)



Date drilled: Drilling contractor: Hydrogeologist 25/02/2009 Brewis J. Pretorius Drilling diameter: 165 mm Final blow yield: DRY Collar height: -1.23 m





Project: Medupi Hydrogeological Study

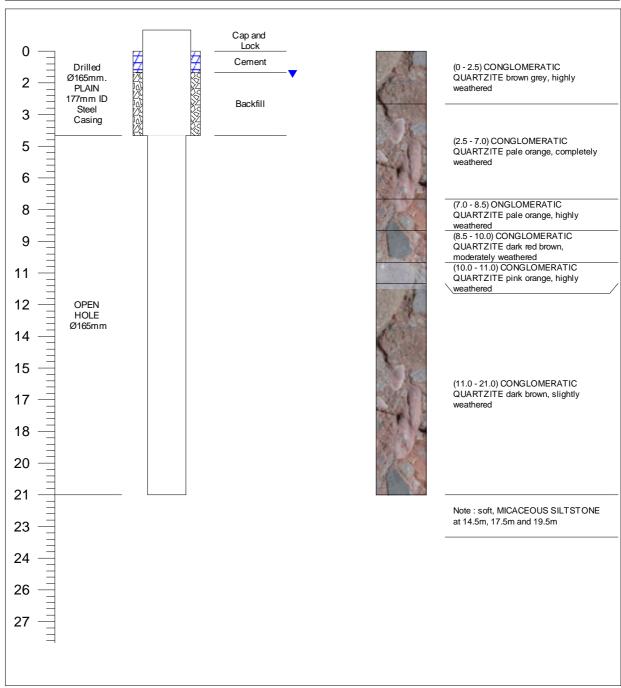
Location: Medupi Power Station

Latitude: -57970 S Project No.:12087

Longitude: 2623325 E Borehole Number

Elevation: 893.62 mamsl GA030

Depth Well Water strike(s)
(m) Construction Water level Lithology Lithology Description (mbgl)



Date drilled: Drilling contractor: Hydrogeologist 25/02/2009 Brewis J. Pretorius Drilling diameter: 165 mm Final blow yield: DRY Collar height: -1.1 m





Project: Medupi Hydrogeological Study

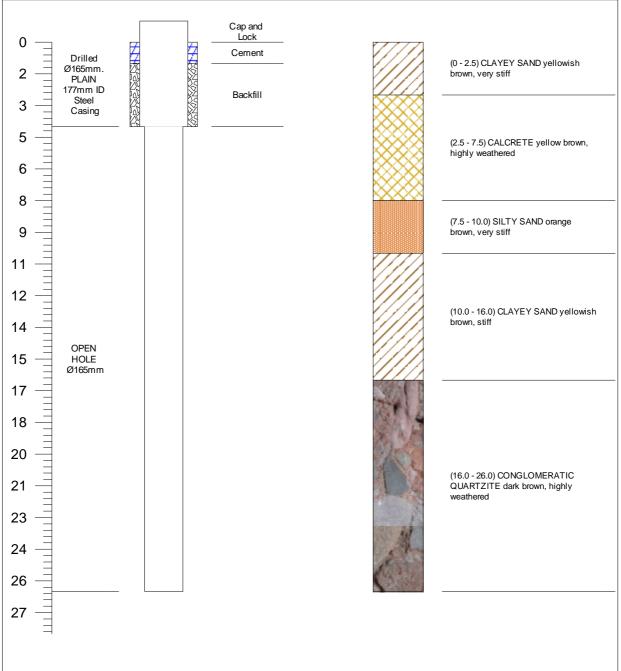
Location: Medupi Power Station

Latitude: -56643 S Project No.:12087

Longitude: 2621984 E Borehole Number

Elevation: 898.67 mamsl GA031

Depth (m) Well (mode) Water strike(s) and Water level (mbgl) Lithology Lithology Description (mbgl)



Date drilled: Drilling contractor: Hydrogeologist 24/02/2009 Brewis J. Pretorius Drilling diameter: 165 mm Final blow yield: DRY Collar height: -1.26m





Project: Medupi Hydrogeological Study

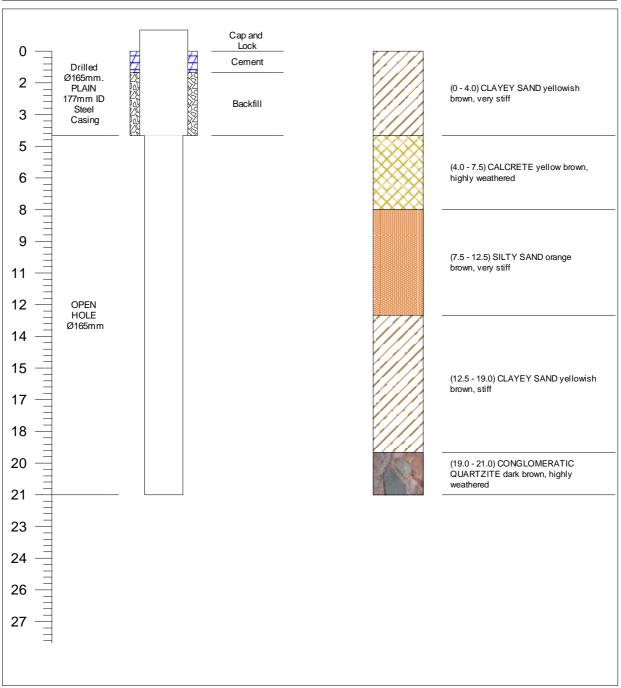
Location: Medupi Power Station

Latitude: -57430 S Project No.:12087

Longitude: 2621610 E Borehole Number

Elevation: 895.09 mamsl GA032

Depth Well and Water level Lithology Lithology Description (m) Construction (mbgl)	Depth (m)	Well Construction		Lithology	Lithology Description	
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Date drilled: Drilling contractor: Hydrogeologist 24/02/2009 Brewis J. Pretorius Drilling diameter: 165 mm Final blow yield: DRY Collar height: -1.2 m





Project: Medupi Hydrogeological Study

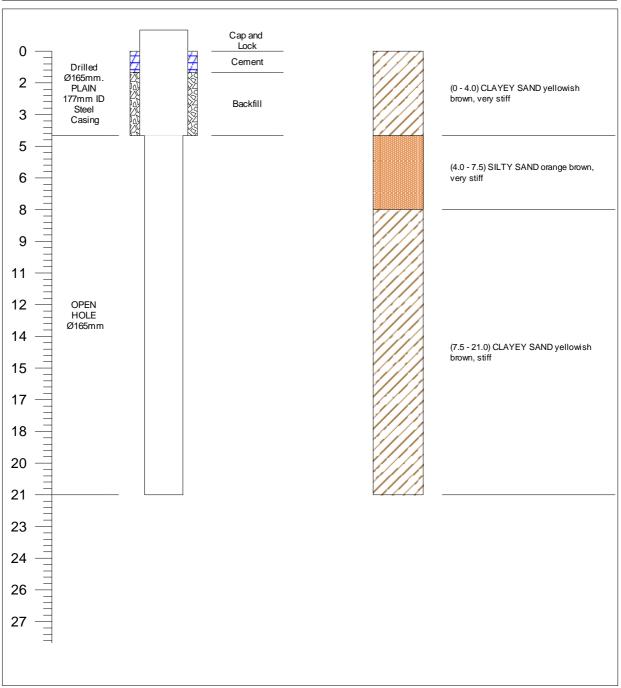
Location: Medupi Power Station

Latitude: -57802 S Project No.:12087

Longitude: 2621821 E Borehole Number

Elevation: 894.36 mamsl GA033

Depth (m)	Well Construction	Water strike(s) and Water level (mbgl)	Lithology	Lithology Description
` '		(IIIbgi)		



Date drilled: Drilling contractor: Hydrogeologist 23/02/2009 Brewis J. Pretorius Drilling diameter: 165 mm Final blow yield: DRY Collar height: -1.27m





Project: Medupi Hydrogeological Study

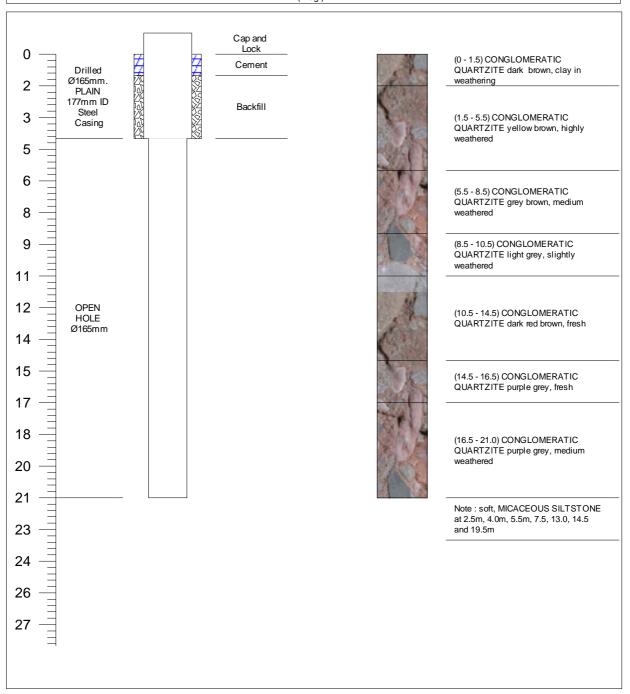
Location: Medupi Power Station

Latitude: -58217 S Project No.:12087

Longitude: 2622071 E Borehole Number

Elevation: 899 mamsl GA034

Depth Well Water strike(s)
(m) Construction (mbgl) Lithology Lithology Description



Date drilled: Drilling contractor: Hydrogeologist 19/02/2009 Brewis J. Pretorius Drilling diameter: 165 mm Final blow yield: DRY Collar height: -1.25 m





Project: Medupi Hydrogeological Study

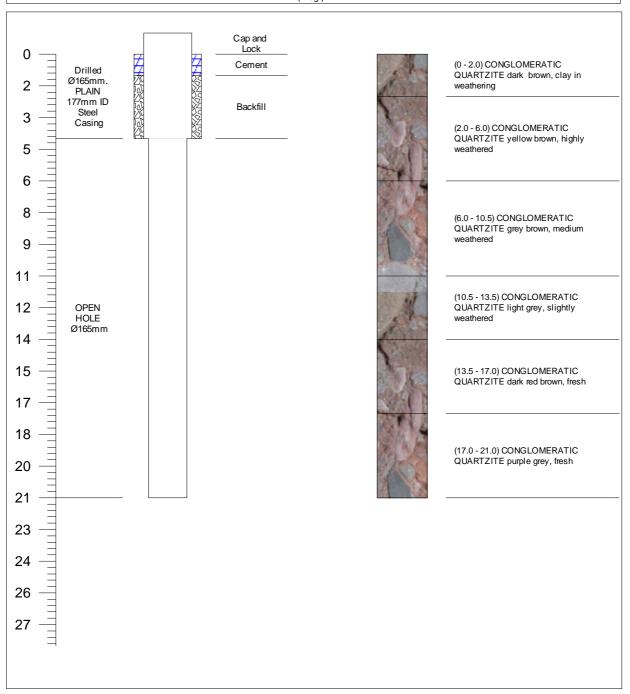
Location: Medupi Power Station

Latitude: -58291 S Project No.:12087

Longitude: 2622492 E Borehole Number

Elevation: 902 mamsl GA035

Depth Well Water strike(s)
(m) Construction (mbgl) Lithology Lithology Description



Date drilled: Drilling contractor: Hydrogeologist 19/02/2009 Brewis J. Pretorius Drilling diameter: 165 mm Final blow yield: DRY Collar height: -1.18 m





Project: Medupi Hydrogeological Study

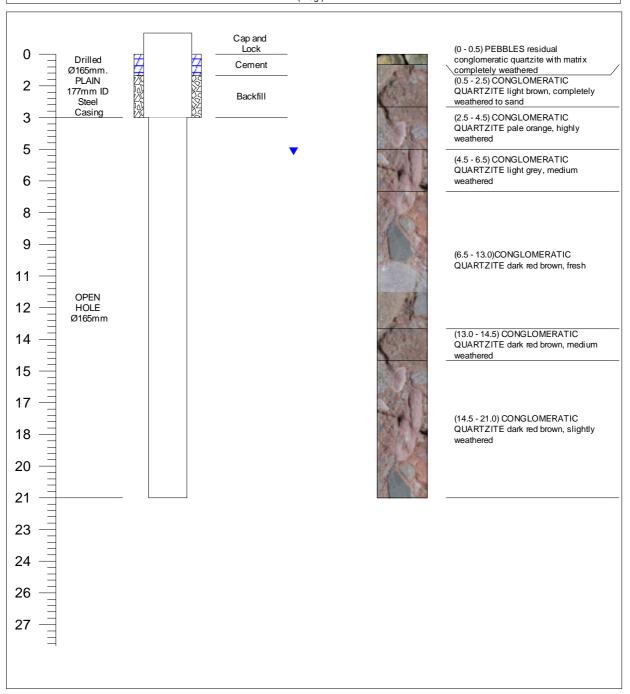
Location: Medupi Power Station

Latitude: -58372 S Project No.:12087

Longitude: 2622964 E Borehole Number

Elevation: 899 mamsl GA036

Depth Well Water strike(s)
(m) Construction (mbgl) Lithology Lithology Description



Date drilled: Drilling contractor: Hydrogeologist 24/02/2009 Brewis J. Pretorius Drilling diameter: 165 mm Final blow yield: 0.1l/s Collar height: -1.2 m



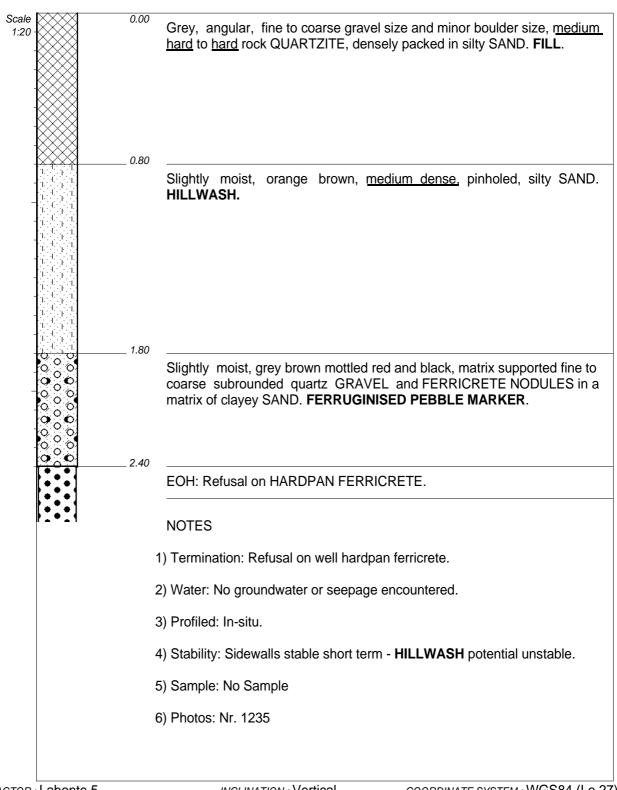


# MEDUPI POWER STATION RAIL YARD

HOLE No: TP23
Sheet 1 of 1

JOB: RG014169

#### **GEOTECHNICAL INVESTIGATION**



CONTRACTOR: Labonte 5
MACHINE: CAT 428D
DRILLED BY: Sonty
PROFILED BY: I Deale
TYPE SET BY: EM

SETUP FILE: ROCKLAND.SET

INCLINATION : Vertical DIAM : 0,7m

DATE: DATE: 10 November 2014

DATE: 09/12/2014 10:46

TEXT: C\WP51\PROFILES\ROC057.TXT

COORDINATE SYSTEM: WGS84 (Lo 27)

X-COORD: 56085 Y-COORD: -2623674

HOLE No: TP23

MEDUPI RAIL YARD TP23



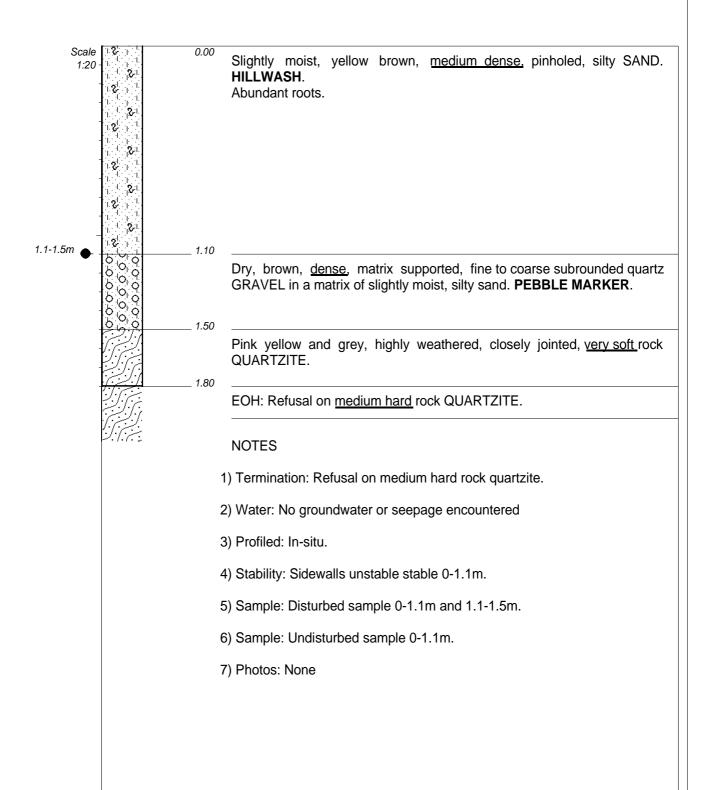


# MEDUPI POWER STATION RAIL YARD

HOLE No: TP07 Sheet 1 of 1

JOB: RG014169

#### **GEOTECHNICAL INVESTIGATION**



CONTRACTOR: Labonte 5
MACHINE: CAT 428D
DRILLED BY: Sonty
PROFILED BY: I Deale
TYPE SET BY: EM

SETUP FILE: ROCKLAND.SET

INCLINATION : VERTICAL DIAM : 0,7m

DATE: DATE: 28 October 2014

DATE: 09/12/2014 10:45

TEXT: C\WP51\PROFILES\ROC057.TXT

COORDINATE SYSTEM: WGS84 (Lo 27)

X-COORD: 56075 Y-COORD: -2623725

HOLE No: TP07

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North America + 1 800 275 3281
South America + 56 2 2616 2000

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